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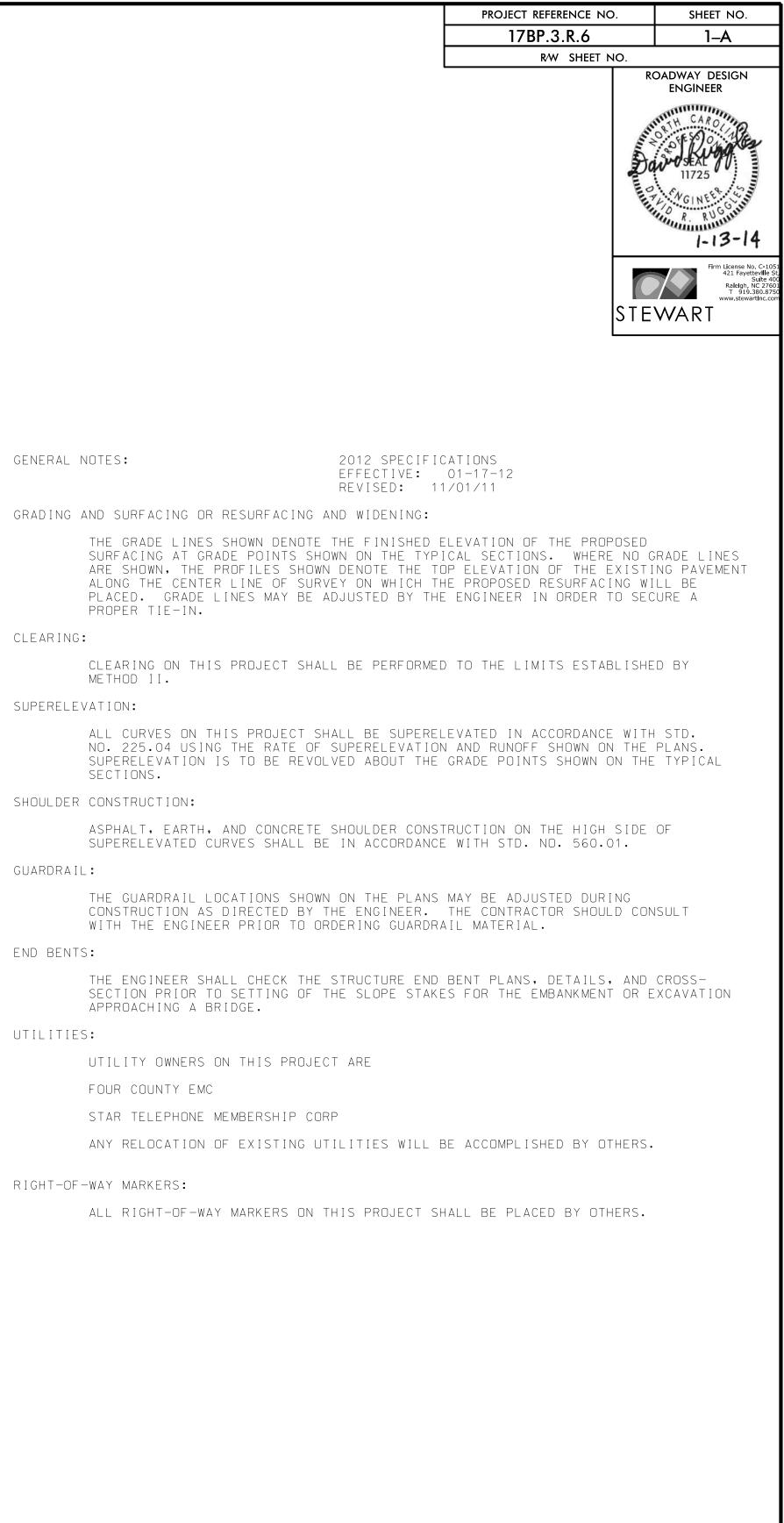
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS

2012 ROADWAY ENGLISH STANDARD DRAWINGS	GENERAL
The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch - N. C. Department of Transportation - Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans:	GRADING
STD.NO. TITLE DIVISION 2 - EARTHWORK 200.03 Method of Clearing - Method II 225.02 Guide for Grading Subgrade - Secondary and Local 225.04 Method of Obtaining Superelevation - Two Lane Pavement DIVISION 3 - PIPE CULVERTS	
 300.01 Method of Pipe Installation DIVISION 4 - MAJOR STRUCTURES 422.10 Reinforced Bridge Approach Fills DIVISION 5 - SUBGRADE, BASES AND SHOULDERS 560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I 	CLEARIN
DIVISION 8 - INCIDENTALS 806.01 Concrete Right-of-Way Marker 806.02 Granite Right-of-Way Marker 840.00 Concrete Base Pad for Drainage Structures 840.25 Anchorage for Frames - Brick or Concrete or Precast 840.29 Frames and Narrow Slot Flat Grates 840.35 Traffic Bearing Grated Drop Inlet - for Cast Iron Double Frame and Grates	SUPEREL
 840.46 Traffic Bearing Precast Drainage Structure 840.66 Drainage Structure Steps 846.01 Concrete Curb, Gutter and Curb & Gutter 846.04 Drop Inlet Installation in Shoulder Berm Gutter 862.01 Guardrail Placement 862.02 Guardrail Placement 	SHOULDE
 862.02 Guardrail Installation 862.03 Structure Anchor Units 876.02 Guide for Rip Rap at Pipe Outlets 	GUARDRA
	end ben
Disclaimer: This coordinate list is provided for the convenience of interested contractors and is intended for use during the project bidding process only. Coordinates are localized to this particular project and any conversion to state grid coordinates or other formats will be the resonsibility of the recipient. While every effort has been made to provide up-to-date, accurate information, NCDOT makes on express guarantee as to the validity or potential for revision of this information prior to project letting.	UTILITI

-L- CENTERLINE COORDINATE LIST

Point#	Chain	Station	Norhting(Y)	Easting (X)
1	L	10+00.00	498123.8981	2114288.7971
2	L	11+00.00	498070.3634	2114373.2603
3	Ĺ	12+00.00	498019.9063	2114459.5339
4	L	13+00.00	497981.3762	2114551.7259
5	L	14+00.00	497955.9805	2114648.3643
6	L	15+00.00	497944.2082	2114747.5881
7	L	16+00.00	497946.2861	2114847.4861
8	L	17+00.00	497962.1742	2114946.1345
9	L	18+00.00	497991.5664	2115041.6334
10	L	19+00.00	498029.8546	2115134.0108



C.U.E. = Subsurface Utility Engineering	ng
BOUNDARIES AND PROPERTY:	,
State Line	
County Line	
Township Line	
City Line	
Reservation Line	· · ·
Property Line	
Existing Iron Pin	
Property Corner	×
Property Monument	
Parcel/Sequence Number	
Existing Fence Line	-
Proposed Woven Wire Fence	
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary	
Proposed Wetland Boundary	
Existing Endangered Animal Boundary	
Existing Endangered Plant Boundary	
Known Soil Contamination: Area or Site —	
Potential Soil Contamination: Area or Site —	
BUILDINGS AND OTHER CULT	
Gas Pump Vent or U/G Tank Cap	
Sign	
Well	-
Small Mine	
Foundation	
Area Outline	
Cemetery	
Building	
School	
Church	
Dam	
HYDROLOGY:	
Stream or Body of Water	
Hydro, Pool or Reservoir	
Jurisdictional Stream	s
Buffer Zone 1	
Buffer Zone 2	— — BZ 2 — — —
Flow Arrow	~~~~~
Disappearing Stream	_ >
	-0
Spring	
Spring Wetland	

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

CONVENTIONAL PLAN SHEET SYMBOLS

LROADS:

Gauge	CSX TRANSPORTATION	
al Milepost	⊙ MILEPOST 35	Orchard —
	SWITCH	Vineyard —
doned	_++++_	EXISTIN
antled		MAJOR:
T OF WAY:		Bridge, Tunr
Control Point	•	Bridge Wing
Right of Way Marker	\bigtriangleup	MINOR:
Right of Way Line		Head and
Right of Way Line		Pipe Culver
l Right of Way Line with Pin and Cap Marker		Footbridge
l Right of Way Line with rete or Granite Marker		Drainage B Paved Ditch
Control of Access	(<u>Ĉ</u>),	Storm Sew
Control of Access		Storm Sew
Easement Line	———— E ———	
Temporary Construction Easement –	E	UTILITII
Temporary Drainage Easement	TDE	POWER:
Permanent Drainage Easement —	PDE	Existing Pov
Permanent Drainage / Utility Easement	DUE	Proposed P
Permanent Utility Easement	PUE	Existing Join
Temporary Utility Easement	TUE	Proposed Jo
Aerial Utility Easement	AUE	Power Man
Permanent Easement with	^	Power Line
Pin and Cap Marker	$\langle \bullet \rangle$	Power Trans
S AND RELATED FEATURE	<i>S:</i>	U/G Power
Edge of Pavement		H-Frame Po
Curb		Recorded U
I Slope Stakes Cut		Designated
l Slope Stakes Fill ——————————————————————————————————	F	
l Curb Ramp	CR	TELEPHONE:
Metal Guardrail ————————————————————————————————————	TT	Existing Tel
l Guardrail —————	<u> </u>	Proposed To
Cable Guiderail ————	<u> </u>	Telephone
l Cable Guiderail	<u> </u>	Telephone
Symbol	\odot	Telephone
nt Removal	$\times\!\!\times\!\!\times\!\!\times\!\!\times$	Telephone
TATION:		U/G Teleph
ree	£	Recorded U
hrub	Ę	Designated
	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	Recorded U
ine		Designated
		Recorded U

Orchard	භි	භි	භි	යි	
Vineyard		Viney	ard		

# NG STRUCTURES:

MAJOR:	
Bridge, Tunnel or Box Culvert [	CONC
Bridge Wing Wall, Head Wall and End Wall –	) CONC WW (
MINOR: Head and End Wall	CONC HW
Pipe Culvert	
Footbridge ————————————————————————————————————	
Drainage Box: Catch Basin, DI or JB	СВ
Paved Ditch Gutter	
Storm Sewer Manhole	S
Storm Sewer	s

# TES:

POWER:	
Existing Power Pole	$\bullet$
Proposed Power Pole	6
Existing Joint Use Pole	
Proposed Joint Use Pole	-0-
Power Manhole	P
Power Line Tower	$\square$
Power Transformer	$\swarrow$
U/G Power Cable Hand Hole	Η _H
H–Frame Pole	••
Recorded U/G Power Line	P
Designated U/G Power Line (S.U.E.*) ——— –	— — P — — — —

## DNE:

Existing Telephone Pole	-•-
Proposed Telephone Pole	-0-
Telephone Manhole	T
Telephone Booth	3
Telephone Pedestal	T
Telephone Cell Tower	, Ť,
U/G Telephone Cable Hand Hole	HH
Recorded U/G Telephone Cable	T
Designated U/G Telephone Cable (S.U.E.*) $-$	T
Recorded U/G Telephone Conduit	TC
Designated U/G Telephone Conduit (S.U.E.* <del>)</del>	TC
Recorded U/G Fiber Optics Cable	T F0
Designated U/G Fiber Optics Cable (S.U.E.* <del>)</del>	— — — T FO— — ·

S	

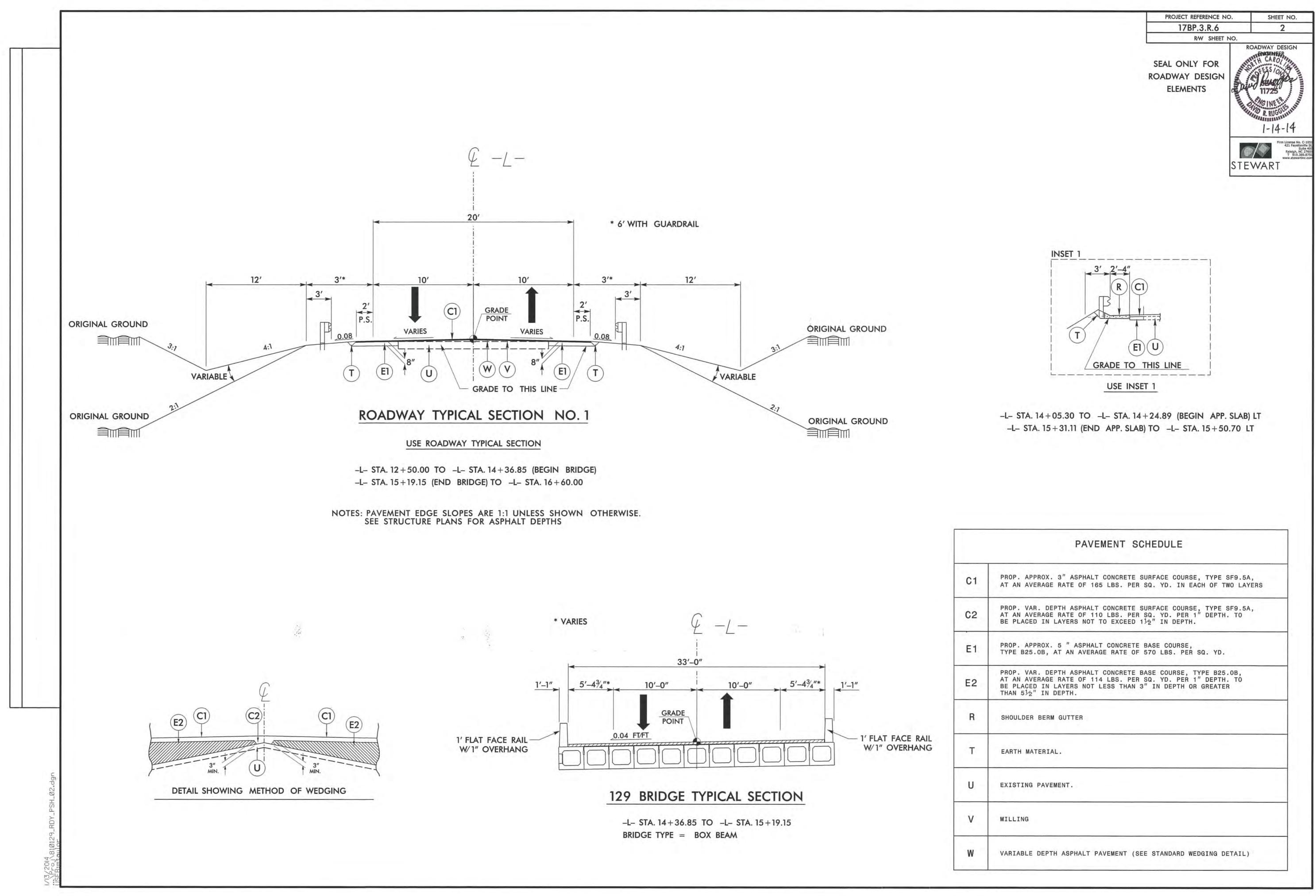
	R/W SHEET	NO.
	FIRM License No. C- 421 Fayettevill Suite Raleigh, NC 27 T 919-380.6 www.stewartinc.	e St, 400 7601 3750
WATER:		•
Water Manhole		Ŵ
Water Meter		$\bigcirc$
Water Valve		$\otimes$
Water Hydrant		ď. C
Recorded U/G Water Line		
Designated U/G Water Lin	e (S.U.E.*)	ww
Above Ground Water Line	· · ·	A/G Water
τ\/.		
TV:		
iv salellie Dish		
I v redesidi		
TV Tower		$\bigotimes$
U/G TV Cable Hand Hole		HH
Recorded U/G TV Cable -		
Designated U/G TV Cable		
Recorded U/G Fiber Optic		
Designated U/G Fiber Opt	ic Cable (S.U.E.*)—	— — — TV FO— — —
GAS:		
Gas Valve		$\diamond$
Gas Meter		$\diamondsuit$
Recorded U/G Gas Line –		
Designated U/G Gas Line		
Above Ground Gas Line -		A/G Gas
SANITARY SEWER:		
Sanitary Sewer Manhole –		$\oplus$
Sanitary Sewer Cleanout –		$(\neq)$
U/G Sanitary Sewer Line –		SS
Above Ground Sanitary Se	ewer	A/G Sanitary Sewer
Recorded SS Forced Main	Line	FSS
Designated SS Forced Mai	in Line (S.U.E.*) —	— — — — FSS — — — –
MISCELLANEOUS:		
Utility Pole		•
Utility Pole with Base ——		
Utility Located Object —		$\odot$
Utility Traffic Signal Box —		S
Utility Unknown U/G Line		?UTL
U/G Tank; Water, Gas, Oil		
Underground Storage Tan	k, Approx. Loc. ——	
A/G Tank; Water, Gas, Oil		
Geoenvironmental Boring		$\bigotimes$
U/G Test Hole (S.U.E.*) —		
Abandoned According to	Utility Records ——	AATUR
End of Information		E.O.I.

PROJECT REFERENCE NO.

17BP.3.R.6

SHEET NO.

1B



BRIC	DGE 810129				
			STATE OF NOR DIVISION OF	HIGHWAYS	
	ltem Number	Sec	Quantity	IES FOR CONTRACT - Unit	Description
		#	Quantity	Onit	Description
	0000100000-N	800	Lump Sum		MOBILIZATION
	0000400000-N	801	Lump Sum		CONSTRUCTION SURVEYING
	002900000-N	SP	Lump Sum		REINF BRG APPR *********
	0042000000 N	226			(-L- STA. 14+78.00)
	0043000000-N	226	Lump Sum		GRADING
	0335200000-Е	305	14	LF	15" DRAINAGE PIPE
	133000000-Е	607	210	SY	INCIDENTAL MILLING
	148900000-Е	610	190	TON	ASP CONC BASE CRS B25.0B
	1525000000-E	610	210	TON	ASP CONC SURF CRS SF9.5A
		600	22		
	157500000-Е	620	23	TON	ASP FOR PLANT MIX
	228600000-N	840	1	EA	MASNRY DRAINAGE STRUCT
	2355000000-N	840	1	EA	FRAME W/GRATE 840.29 STD
	255600000-е	846	39.2	LF	SHOULDER BERM GUTTER
	303000000-E	862	50	LF	STL BM GUARDRAIL
	3150000000-N	862	3	EA	ADDIT GUARDRAIL POSTS
	212000000-11	002	5	LA	ADDIT GOARDRAIL POSTS
	3215000000-N	862	4	EA	GR ANCHOR TYPE III
	327000000-N	862	4	EA	GR ANCHOR TYPE 350
	364900000-E	876	2	TON	RIP RAP, CLASS B
	365600000-е	876	200	SY	GEOTEXTILE FOR DRAINGE
	440000000-E	1110	322	SF	WORK ZONE SIGNS (STAT)
			—		()

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# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# SUMMARY OF QUANTITIES

ltem Number	Sec #	Quantity	Unit	Description	ltem Number
441000000-E	1110	94	SF	WORK ZONE SIGNS (BARR)	6071020000-E
4445000000-E	1205	64	LF	BARRICADES (TYPE III)	608400000-E
481000000-E	1251	3280	LF	PAINT PVMT MARKINGS 4"	609000000-E
490000000-N	1605	11	EA	PERM RAISED PVMT MARKERS	609300000-E
600000000-E	1610	950	LF	TEMPORARY SILT FENCE	609600000-E
600600000-E	1610	65	TON	EROS CONTRL STONE CL A	
600900000-E	1610	15	TON	EROS CONTRL STONE CL B	610800000-Е
601200000-E	1615	10	TON	SEDIMENT CONTROL STONE	611700000-N
601500000-E	1620	0.5	ACR	TEMPORARY MULCHING	803500000-N
601800000-E	1620	100	LB	SEED FOR TEMP SEEDING	8112730000-N
602100000-E	1622	0.5	TON	FERT FOR TEMP SEEDING	812100000-N
602400000-E	SP	200	LF	TEMPORARY SLOPE DRAINS	818200000-E
602900000-E	1630	100	LF	SAFETY FENCE	821000000-N
603000000-E	1631	20	CY	SILT EXCAVATION	821700000-E
603600000-E	SP	1250	SY	MATTING FOR EROS CONTROL	8364000000-E
603800000-E	1632	35	SY	PERM SOIL REINF MAT	839300000-N
604200000-E	SP	25	LF	1/4" HARDWARE CLOTH	850500000-E
604800000-E	SP	210	SY	FLOAT TURBIDITY CURTAIN	860800000-E
6071012000-E	1660	140	LF	COIR FIBER WATTLE	865700000-N

8753100000-E

PROJECT REFERENCE NO.	SHEET NO.
17BP.3.R.6	3

Sec #	Quantity	Unit	Description
SP	10	LB	POLYACRYLAMIDE (PAM)
1661	3	LB	SEEDING AND MULCHING
1662	50	ACR	SEED FOR REPAIR SEEDING
1665	0.25	LB	FERT FOR REPAIR SEEDING
1667	50	TON	SEED FOR SUPP SEEDING
SP	1.5	LB	FERTILIZER TOPDRESSING
SP	4	TON	RESPONSE FOR EROS CONTROL
402	1	EA	REMV EXIST STR ********* (-L- STA. 14+78.00)
450	1	LS	PDA TESTING
412	1	EA	UNCL STR EXCAV STA ***** (-L- STA. 14+78.00)
420	34	LS	CLASS A CONCRETE (BRIDGE)
422	1	СҮ	BRG APPR SLAB ********** (-L- STA. 14+78.00)
425	5624	LS	REINF STEEL (BRIDGE)
450	630	LB	HP12X53 PILES
450	6	LF	PILE REDRIVES
460	160	EA	VERT CONC BARRIER RAIL
876	200	LF	RIP RAP II (2'-0")
430	1	TON	ELASTOMERIC BEARINGS
430	880	LS	3'-0"X 2'-9" PRESTR BOX BEAM

TION 12 + 50.00 15 + 19.15 PROJ O REPLA	0 JECT 1 ACE T	- - TOTAL	STA -L- STA. 14 - -L- STA. 16 - .: OIL ON B(	TION + 36.85 + 60.00				ЕМВА	ANK. 6 2 8		DR BORI 92 8 10	ROW 2 5 5								
12 + 50.00 15 + 19.15 PROJ GRA GRA	ACE T	TOTAL TOTAL	L- STA. 14 - L- STA. 16 - .: OIL ON BO	+ 36.85 + 60.00		CAV. 4 14 18		+ 9 2	% 6 2 8		9: 8 10	2 00 5			/ASTE					
I5 + 19.15 PROJ O REPLA GRA GRA	ACE T	TOTAL TOTAL	L_ STA. 16 -	+ 60.00		14		2	2		8 10	00								
O REPLA GRA	ACE T	TOTAL									5	5								
(LT,RT, OR CL)	ND T	TOTAL				18														
(LT,RT, OR CL)				VIION																
(LT,RT, OR		STRUCTURE NO.	NOITEV:	VIION	ZO															
	V 0401 0401	₽ 0402	р Р 127.71	INVERT ELEVATION	INVERT ELEVATION	SLOPE CRITICAL	12″		CSP, (			PE E, or P		DO NOT USE RCP	12″ 12″	15″ • • • • • • • • • • • • • • • • • • •	.064	24"		30"
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		15)	BRIDGE	(14+36.85)																
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							2											ΤΟΤΑ	_ =	
	R WIDT = DIS ⁻ /IDTH ( IMPAC ⁻ SATING BEG. 13 + 59 13 + 59 13 + 59 13 + 59 13 + 59	R WIDTH = DISTANC /IDTH OF FL IMPACT ATT GATING IMP/ BEG. STA. 13 + 59.40 13 + 54.15 DGE (15 + 19.	R WIDTH = DIS = DISTANCE FRC /IDTH OF FLARE I IMPACT ATTENUA GATING IMPACT A BEG. STA. 13+59.40	R WIDTH =       DISTANCE FROM LAST 3         =       DISTANCE FROM LAST 3         /IDTH OF FLARE FROM BEC       IMPACT ATTENUATOR TYPE         GATING IMPACT ATTENUATOR TYPE         BEG. STA.       END         13+59.40       BRIDGE         13+54.15       BRIDGE         IDGE (15+19.15)       15+         IDGE (15+19.15)       16+         DEDL       DEDL	R WIDTH =       DISTANCE FROM LAST SECTION O         =       DISTANCE FROM LAST SECTION O         /IDTH OF FLARE FROM BEGINNING C       IMPACT ATTENUATOR TYPE 350         GATING IMPACT ATTENUATOR TYPE 350         BEG. STA.       END STA.         13+59.40       BRIDGE (14+36.85)         13+54.15       BRIDGE (14+36.85)         IDGE (15+19.15)       15+96.60         IDGE (15+19.15)       16+01.85         DEDUCTIONS FO       DEDUCTIONS FO	CE FROM EDGE OF LANE TO FACE OF GUARD         R WIDTH = DISTANCE FROM EDGE OF TRAVE         = DISTANCE FROM LAST SECTION OF PARALL         //IDTH OF FLARE FROM BEGINNING OF TAPER         IMPACT ATTENUATOR TYPE 350         BEG. STA.       END STA.         I3+59.40       BRIDGE (14+36.85)         I3+54.15       BRIDGE (14+36.85)         DGE (15+19.15)       15+96.60         DGE (15+19.15)       16+01.85         DEDUCTIONS FOR ANCHOR	=       DISTANCE FROM LAST SECTION OF PARALLEL GU         /IDTH OF FLARE FROM BEGINNING OF TAPER TO E         IMPACT ATTENUATOR TYPE 350         BEG. STA.         BEG. STA.         END STA.         LOCATION         13+59.40         BRIDGE (14+36.85)         RT         13+54.15         BRIDGE (14+36.85)         LT         DGE (15+19.15)         16+01.85         LT         DGE (15+19.15)         DGE (15+19.15)	CE FROM EDGE OF LANE TO FACE OF GUARDRAIL.         R WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO         = DISTANCE FROM LAST SECTION OF PARALLEL GUARD         /IDTH OF FLARE FROM BEGINNING OF TAPER TO END         IMPACT ATTENUATOR TYPE 350         BEG. STA.       END STA.         LOCATION         13+59.40       BRIDGE (14+36.85)         I3+54.15       BRIDGE (14+36.85)         LIT         DGE (15+19.15)       15+96.60	CE FROM EDGE OF LANE TO FACE OF GUARDRAIL.         R WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SH         = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL         //IDTH OF FLARE FROM BEGINNING OF TAPER TO END OF COMPACT ATTENUATOR TYPE 350         BEG. STA.       END STA.         I3+59.40       BRIDGE (14+36.85)         RT       I3+54.15         BRIDGE (14+36.85)       LT         DGE (15+19.15)       15+96.60         RT       I         DGE (15+19.15)       16+01.85         LT       I         DEDUCTIONS FOR ANCHOR UNITS       TOTAL	CE FROM EDGE OF LANE TO FACE OF GUARDRAIL. R WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULD = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO E //DTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUAR IMPACT ATTENUATOR TYPE 350 BEG. STA. END STA. LOCATION BEG. STA. END STA. LOCATION 13 + 59.40 BRIDGE (14 + 36.85) RT 81. 13 + 54.15 BRIDGE (14 + 36.85) LT 81. DGE (15 + 19.15) 15 + 96.60 RT 81. DGE (15 + 19.15) 16 + 01.85 LT 81. DGE (15 + 19.15	CE FROM EDGE OF LANE TO FACE OF GUARDRAIL.         R WIDTH =       DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER         =       DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END         IMPACT ATTENUATOR TYPE 350       OF TAPER TO END OF GUARDRAI         BEG. STA.       END STA.         LOCATION       STRAIGHT         13+59.40       BRIDGE (14+36.85)         RT       81.25         DGE (15+19.15)       15+96.60         RT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DEDUCTIONS FOR ANCHOR UNITS       275         TOTAL       50	CE FROM EDGE OF LANE TO FACE OF GUARDRAIL.         R WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK         = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF CONTROL         IMPACT ATTENUATOR TYPE 350         BEG. STA.         END STA.         END STA.         END STA.         LOCATION         STRAIGHT         13+59.40         BRIDGE (14+36.85)         RT       81.25         13+54.15       BRIDGE (14+36.85)         LOCE (15+19.15)       15+96.60         RT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DEDUCTIONS FOR ANCHOR UNITS       275         TOTAL       50	CE FROM EDGE OF LANE TO FACE OF GUARDRAIL. R WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POI = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL. IMPACT ATTENUATOR TYPE 350 SATING IMPACT ATTENUATOR TYPE 350LENUBEG. STA.END STA.LOCATIONISTANCE FROM BEGINNING OF TAPER TO END OF GUARDRAIL. IMPACT ATTENUATOR TYPE 350BEG. 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LOCATION       STRAIGHT         SHOP       DOUBLE         13+59.40       BRIDGE (14+36.85)         I3+59.40       BRIDGE (14+36.85)         LT       81.25         I3+54.15       BRIDGE (14+36.85)         LT       81.25         DGE (15+19.15)       15+96.60         RT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DEDUCTIONS FOR ANCHOR UNITS       275         DEDUCTIONS FOR ANCHOR UNITS       275</td></t<></td></td<> <td>2E FROM EDGE OF LANE TO FACE OF GUARDRAIL.         R WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.         = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.         /IDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.         /IDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.         IMPACT ATTENUATOR TYPE 350         SATING IMPACT ATTENUATOR TYPE 350         BEG. STA.       END STA.         LOCATION       STRAIGHT         SHOP       DOUBLE         13+59.40       BRIDGE (14+36.85)         RT       81.25         13+54.15       BRIDGE (14+36.85)         LOCATION       81.25         DGE (15+19.15)       15+96.60         RT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.16)       16+01.85         LT       81.25         DEDUCTIONS FOR ANCHOR UNITS       275         TOTAL       325         DEDUCTIONS FOR ANCHOR UNITS       275         ADDITIONAL GUARDRAL POSTS = 3       3    &lt;</td> <td>$\begin{array}{c c c c c c c c c c c c c c c c c c c$</td>	CE FROM EDGE OF LANE TO FACE OF GUARDRAIL. R WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT. = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRA IDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL. IMPACT ATTENUATOR TYPE 350 BEG. 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LOCATION       STRAIGHT         SHOP       DOUBLE         13+59.40       BRIDGE (14+36.85)         I3+59.40       BRIDGE (14+36.85)         LT       81.25         I3+54.15       BRIDGE (14+36.85)         LT       81.25         DGE (15+19.15)       15+96.60         RT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DEDUCTIONS FOR ANCHOR UNITS       275         DEDUCTIONS FOR ANCHOR UNITS       275</td></t<>	CE FROM EDGE OF LANE TO FACE OF GUARDRAIL. R WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT. = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL. IDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL. IMPACT ATTENUATOR TYPE 350 SATING IMPACT ATTENUATOR TYPE 350 BEG. STA. END STA. LOCATION BEG. STA. END STA. 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LOCATION       STRAIGHT         SHOP       DOUBLE         13+59.40       BRIDGE (14+36.85)         I3+59.40       BRIDGE (14+36.85)         LT       81.25         I3+54.15       BRIDGE (14+36.85)         LT       81.25         DGE (15+19.15)       15+96.60         RT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DEDUCTIONS FOR ANCHOR UNITS       275         DEDUCTIONS FOR ANCHOR UNITS       275	2E FROM EDGE OF LANE TO FACE OF GUARDRAIL.         R WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.         = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.         /IDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.         /IDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.         IMPACT ATTENUATOR TYPE 350         SATING IMPACT ATTENUATOR TYPE 350         BEG. STA.       END STA.         LOCATION       STRAIGHT         SHOP       DOUBLE         13+59.40       BRIDGE (14+36.85)         RT       81.25         13+54.15       BRIDGE (14+36.85)         LOCATION       81.25         DGE (15+19.15)       15+96.60         RT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.15)       16+01.85         LT       81.25         DGE (15+19.16)       16+01.85         LT       81.25         DEDUCTIONS FOR ANCHOR UNITS       275         TOTAL       325         DEDUCTIONS FOR ANCHOR UNITS       275         ADDITIONAL GUARDRAL POSTS = 3       3    <	$\begin{array}{c c c c c c c c c c c c c c c c c c c $

# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# RIGHT OF WAY AREA DATA

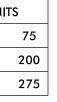
PARCEL NO.	PROPERTY OWNERS NAMES	TOTAL ACREAGE	AREA TAKEN	AREA REMAINING RT.	AREA REMAINING LT.	CONST. EASE.	PERM. DRAIN. EASE.	TEMP. DRAIN. EASE.
1	SHELBY I. & SAMUEL H. JACKSON / CONNIE S. & TERRY F. GAUSE	13.18	_	_	-	0.013	_	-
2	MACK D. & JANICE W. TEW, VANESSA LYNN TEW, AND CARLA T. MARTIN	10.67	_	_	_	0.074	343 sf	_

# SUB-REGIONAL & REGIONALLIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

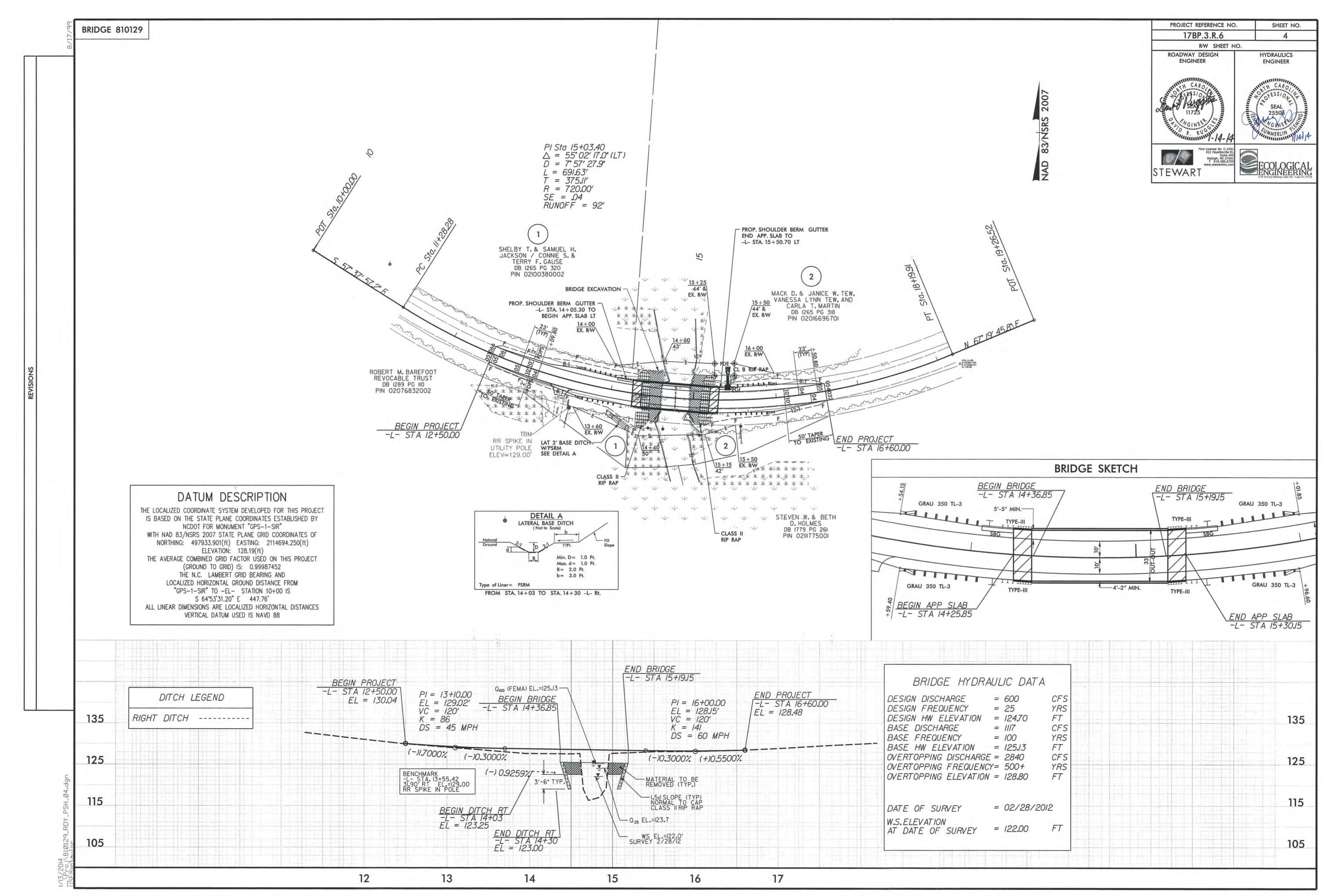
																															 	 -
																		ENDV	VALLS	ES AGE	es R PAY	BE COL. OL.'B')							840.29			
PE OT	HRW	′ISE)						(UI	C NLES:	LASS S NC	III R. DTED	C. Pif Oth	PE IERWI	SE)				C STD. 8 (UN NC	338.01, 838.11 )R 338.80 ILESS DTED RWISE)	QUANTITIES FOR DRAINAGE	* TOTAL L.F. FOI	QUANTITY SHALL BE COL. 'A' + (1.3 X COL'B')	840.02		FRAME, AND   ANDAR	HOOD		0. 840.35	GRATES STD.			
																				5.0')	LII *F	T.	STD. 8/					G.D.I. STD.	WITH TWO		ELBOWS	
)″	3	86″	4	2″	4	8″	12″	15″	18″	24″	30″	36″	42″	48″	PIPE	PIPE	PIPE	CU.	YDS.		A	B ш	OR						ME W		PIPE E	
	.079		.109		.109										side drain	side drain	side drain	R.C.P.	C.S.P.	EACH (0' THRU	THRU 10.0'	)' AND ABOVE	. STD. 840.01	1	TYPE OF	GRATI	:	TRAFFIC BEARING	G.D.I. (N.S.) FRAME		DRAINAGE	
															15″	18″	24"			PER	5.0′	10.0′	C.B.	E	F	G		TR	G.I		15″	
																				1								1	1			 
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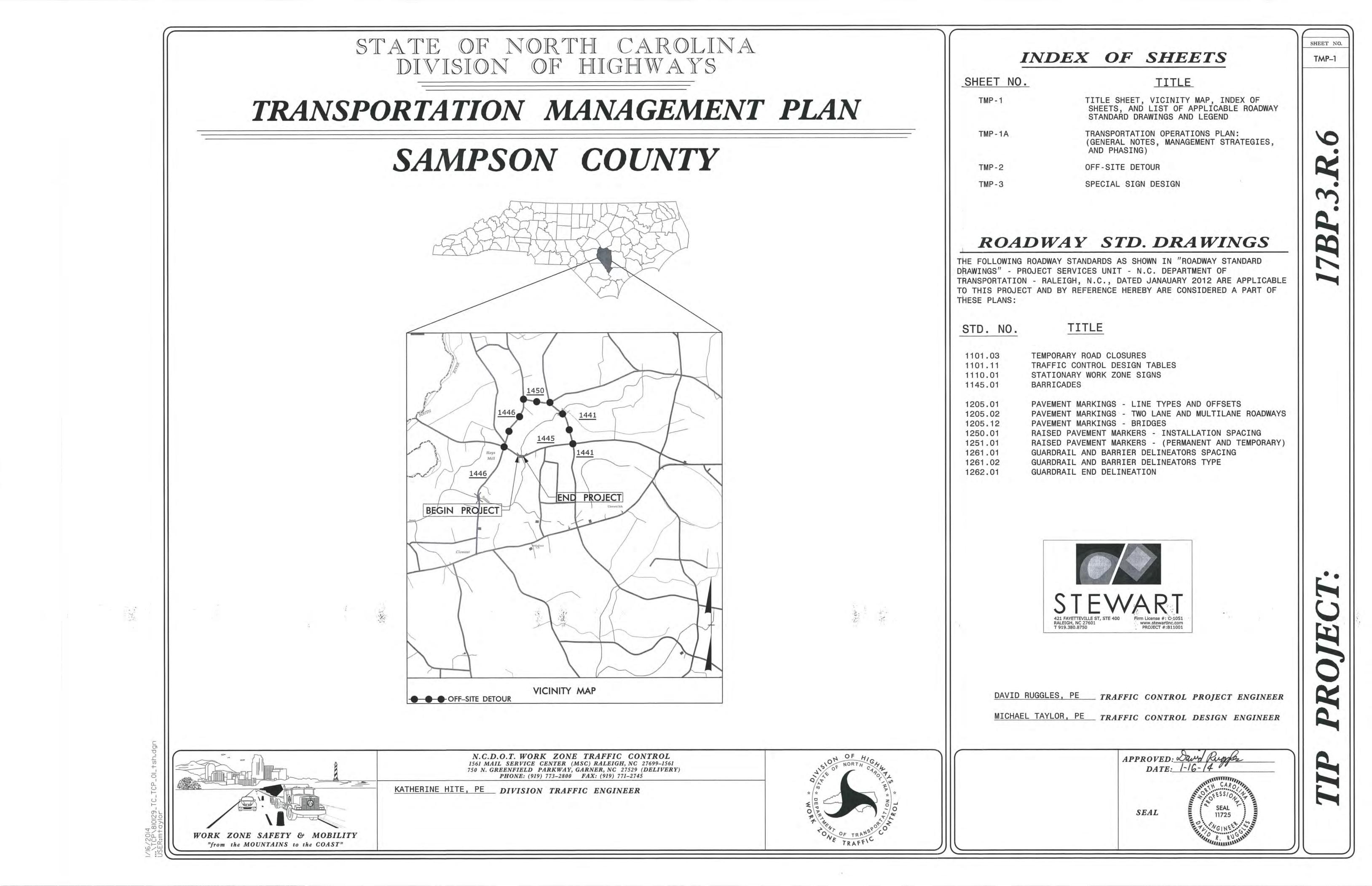
# GUARDRAIL SUMMARY

WARRA	NT POINT	″N″ DIST.	TOTAL	FLARE I	LENGTH	١	w				AN	ICHORS				ATTEN		RSINGLE	REMOVE	REMOVE AND STOCKPILE	
ACH	TRAILING END	FROM E.O.L.	SHOUL. WIDTH	APPROACH END	TRAILING END	APPROACH END	TRAILING END	XI MOD	XI	GRAU M- 350	-350	III CAT-1	VI MOD	BIC	AT–1		350 G N	FACED GUARDRAIL G	EXISTING GUARDRAIL		REMARKS
		5.39′	8.39′		62.5′		1.25′			1		1									
		5.39′	8.39′	62.5′		1.25′				1		1									
		5.39′	8.39′	62.5′		1.25′				1		1									
		5.39′	8.39′		62.5′		1.25′			1		1									
										4		4									



					CT REFERENCE NO	ر. ا	SHEET NO.
					/BP.J.K.O R/W SHEET N	<u> </u>	JA
						1	
					Firm License No. C-105 421 Fayetteville St Suite 400 Raleigh, NC 2760 T 919.380.8750		
				STEWA	www.stewartinc.com		COLOGICAL
				012117		1151	SE Cary Parkway, Suite 101 - Cary NC 27518
1	PAVEN	IEN7	REN	10VAL	, SUM	MAR	Y
	SURVEY LINE	STAT	ION	STATION	LOCATION LT/RT/CL	SY	
	-L-	12 + 6	4.81	12+98.37	CL	81	
	_L_	13+4	9.87	13 + 79.19	CL	73	
					TOTAL:	154	_
SUO		DD	EDM		TER SI	Т <b>ТЛЛ</b> Л	
5110	гг						
	SURVEY LINE	STAT	NUN	STATION	LOCATION	LENGTH (FT)	
	_L_	14 + 0	5.30	14+24.89	LT	19.59	1
	-L-	15 + 3		15 + 50.70	LT	19.59	1
							1
							1
					TOTAL:	39.18	]
					C.B. CATO N.D.I. NARI D.I. DRO G.D.I. GRA G.D.I. (N.S.) GRAT (NAR J.B. JUNO M.H. MAN T.B.D.I. TRAF T.B.J.B. TRAF	CTION BOX HOLE FIC BEARIN FIC BEARIN EMARKS	INLET INLET )
IMPACT ATTENUATOR	SINGLE	REMOVE	REMOVE				





# GENERAL NOTES / LOCAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER TWENTY ONE (21) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

## SIGNING

- DRAWINGS AND TRAFFIC CONTROL PLANS. PLANS.
- IN OPERATION.

B) INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION. STEP 1 INSTALL DETOUR SIGNING AS SHOWN ON SHEET TMP-2 IN ACCORDANCE WITH C) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD ROADWAY STANDARD DRAWING 1101.03, SHEET 1 OF 9. SIGNS SHALL BE COVERED IF DETOUR IS NOT OPENED WITHIN 3 DAYS OF SIGN INSTALLATION. PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL STEP 2 INSTALL BARRICADES AND CLOSE SR 1445 (HAYES MILL ROAD) TO THROUGH TRAFFIC AS SHOWN ON TMP-2 AND IN ACCORDANCE WITH ROADWAY STANDARD DRAWING 1101.03, SHEET 1 OF 9. D) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT STEP 3 PERFORM THE FOLLOWING WORK WITHIN THE ROAD CLOSURE: - REMOVE THE EXISTING STRUCTURE. COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN - CONSTRUCT THE PROPOSED STRUCTURE. OPERATION. - CONSTRUCT THE PROPOSED ROADWAY SECTION -L- UP TO AND INCLUDING THE FINAL LAYER OF SURFACE, THE FINAL PAVEMENT MARKINGS, AND THE FINAL E) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN. PAVEMENT MARKERS FROM -L- STA. 12+50 TO -L- STA. 16+60. STEP 4 REMOVE ALL ROAD CLOSURE SIGNS AND BARRICADES AND OPEN SR 1445 (HAYES MILL ROAD) TO THROUGH TRAFFIC.

TRAFFIC CONTROL DEVICES

F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

PAVEMENT MARKINGS AND MARKERS

- G) INSTALL PAVEMENT MARKINGS ON THE FINAL SURFACE AS FOLLOWS: MARKINGS ROAD NAME SR 1445 (HAYES MILL ROAD) PAINT
- H) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.

MARKERS RAISED

> APPROVED Tand Rugges DATE: 1-16-1 SEAL

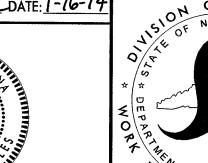
PROJ. REFERENCE NO.	SHEET NO.
17BP.3.R.6	TMP-1A

# MANAGEMENT **STRATEGIES**

CLOSE SR 1445 (HAYES MILL ROAD) TO THROUGH TRAFFIC BETWEEN SR 1441 AND SR 1446. DIRECT THROUGH TRAFFIC TO OFF SITE DETOUR. MAINTAIN LOCAL TRAFFIC.

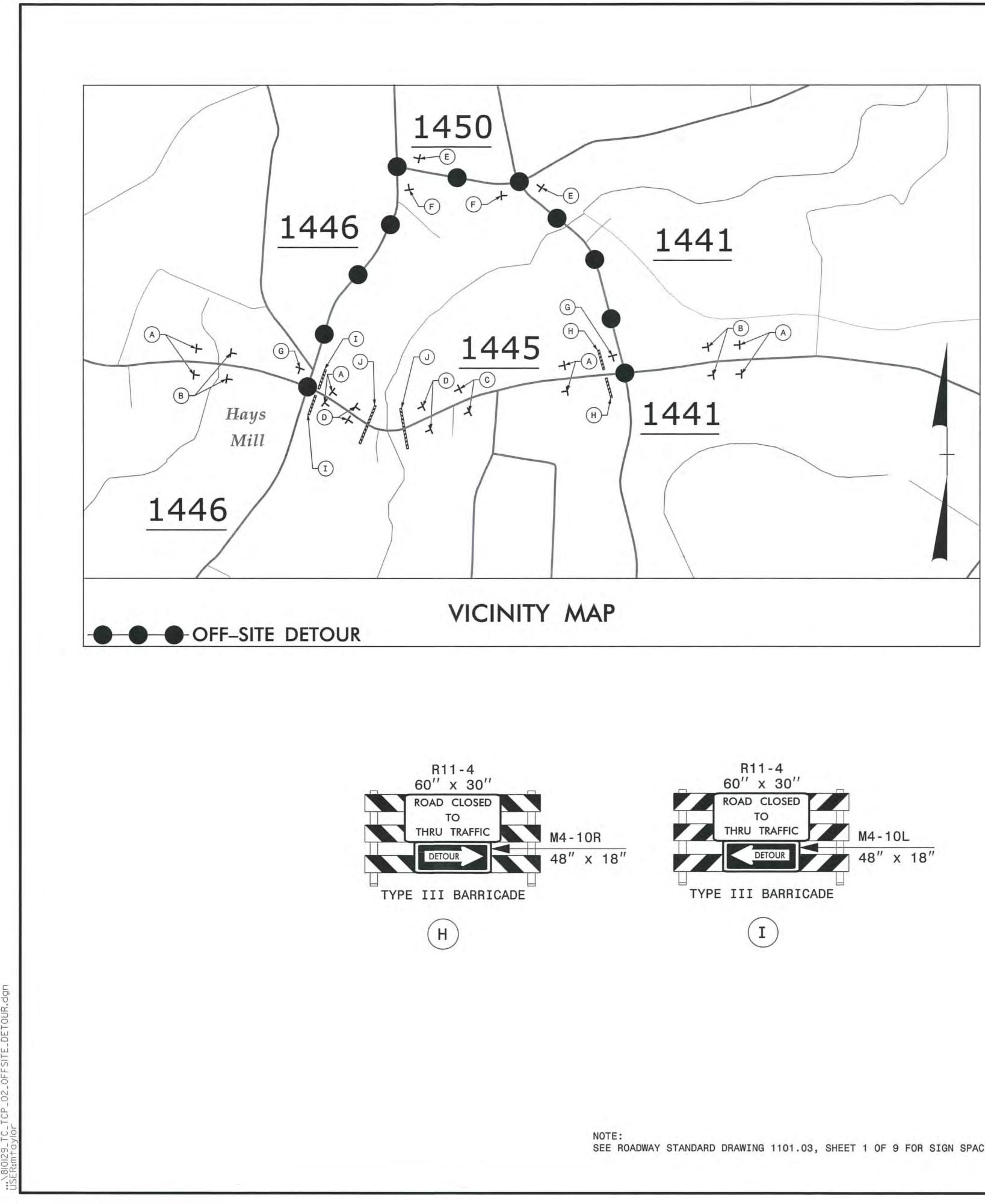
# **PHASING**

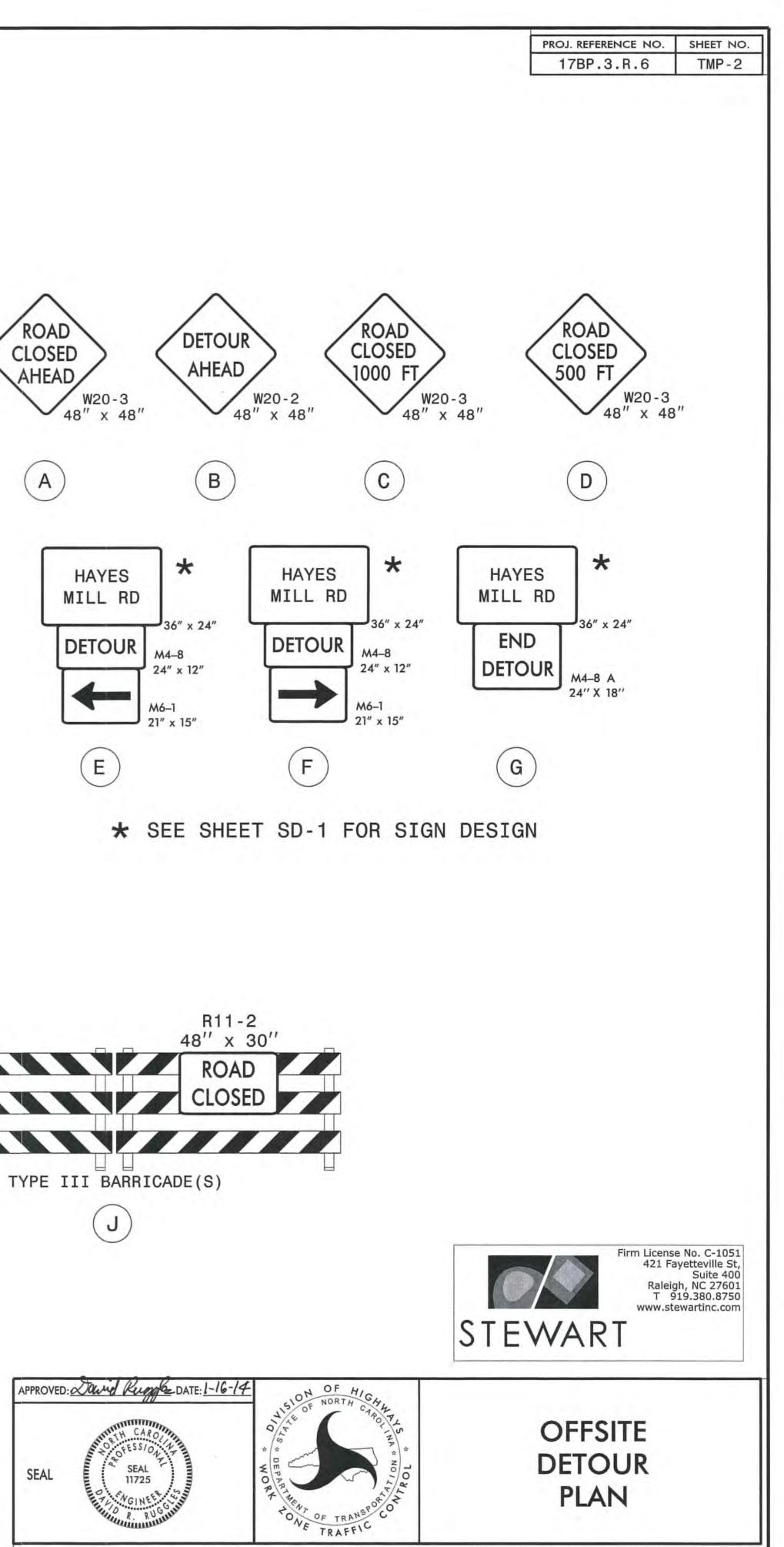






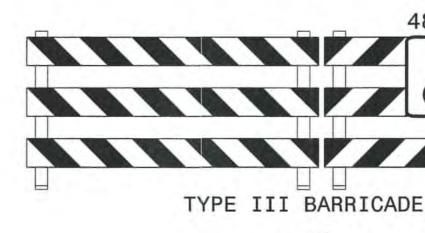
NOTES Å. PHASING

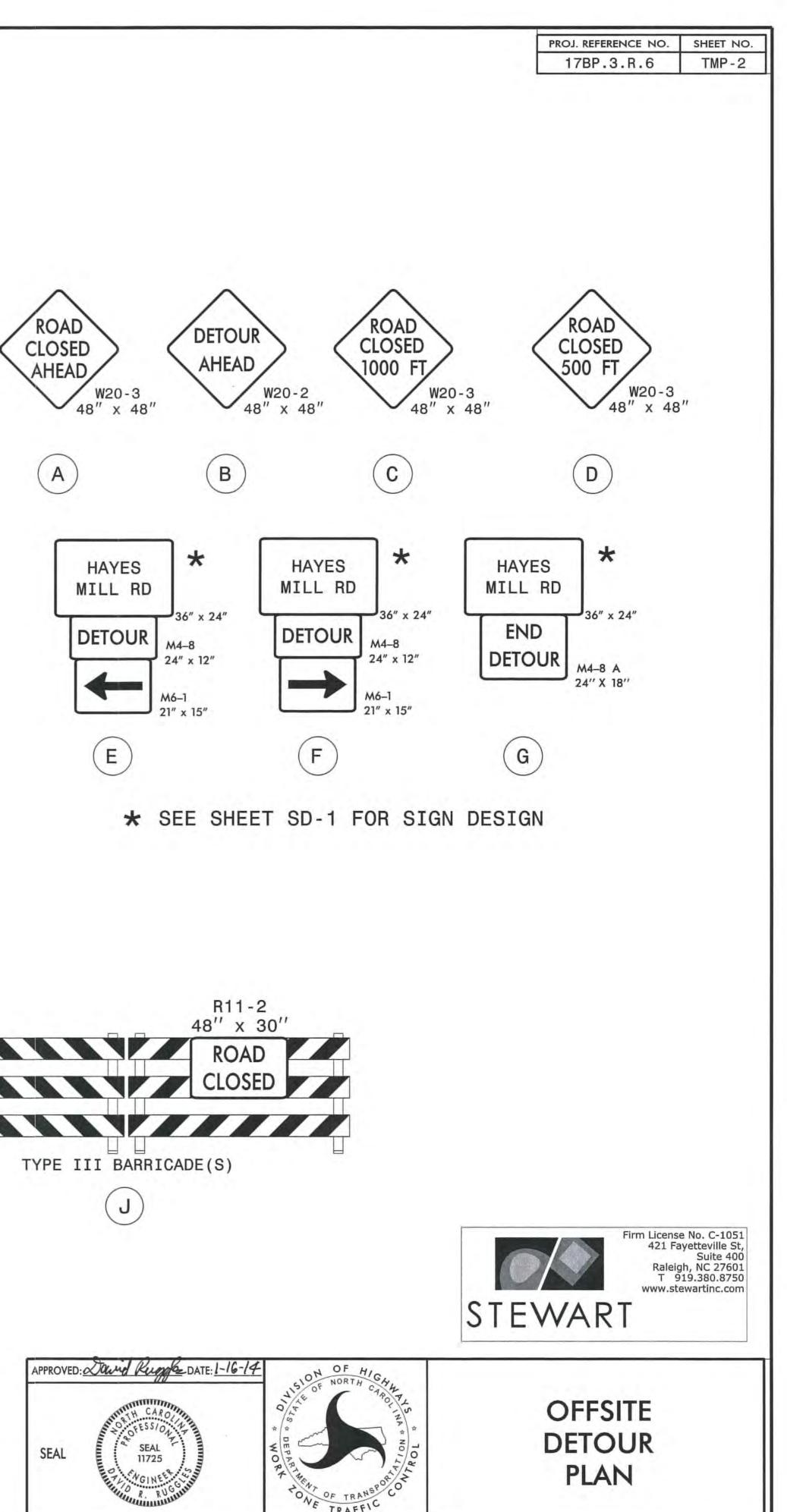


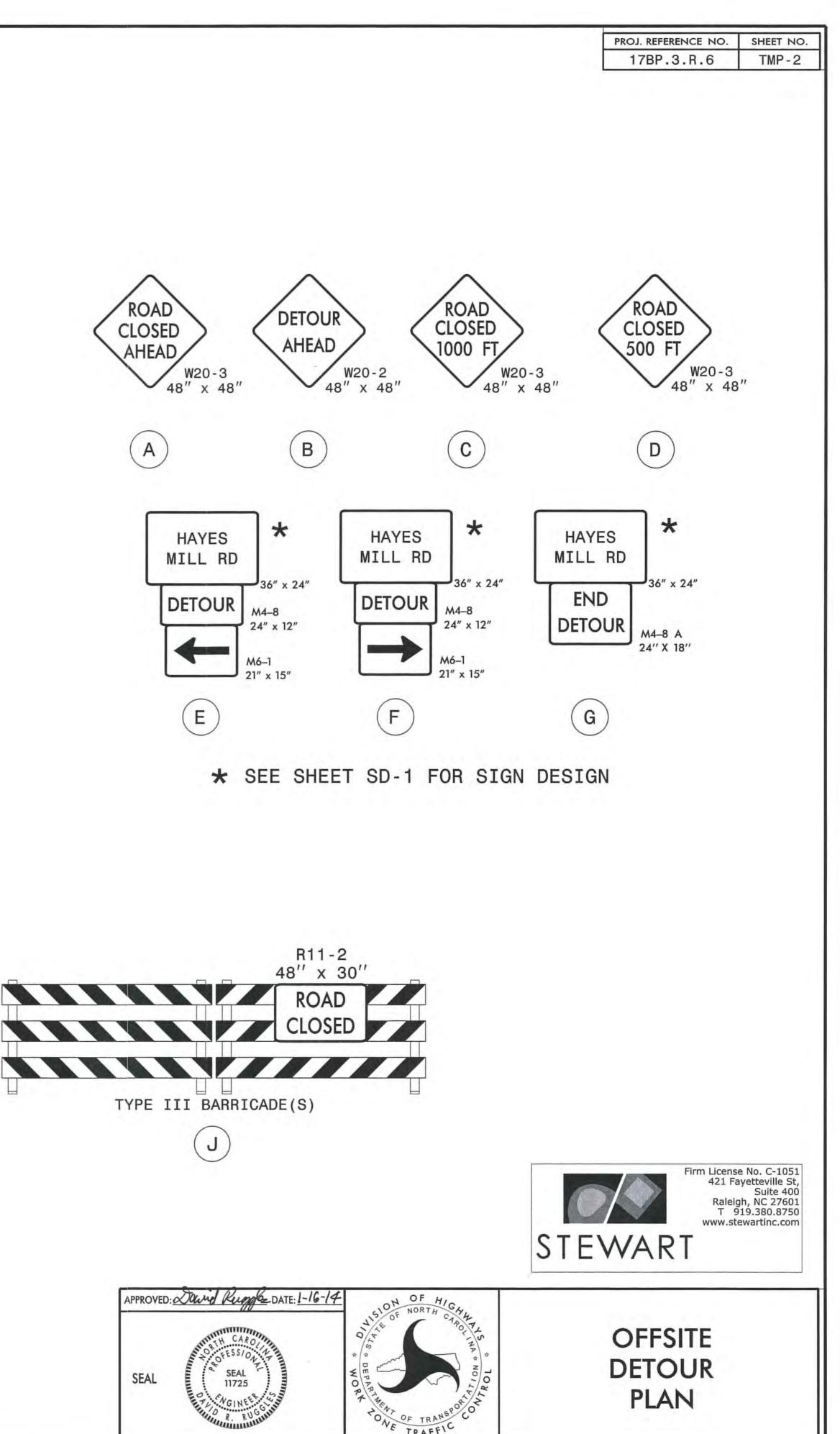


SEE ROADWAY STANDARD DRAWING 1101.03, SHEET 1 OF 9 FOR SIGN SPACING



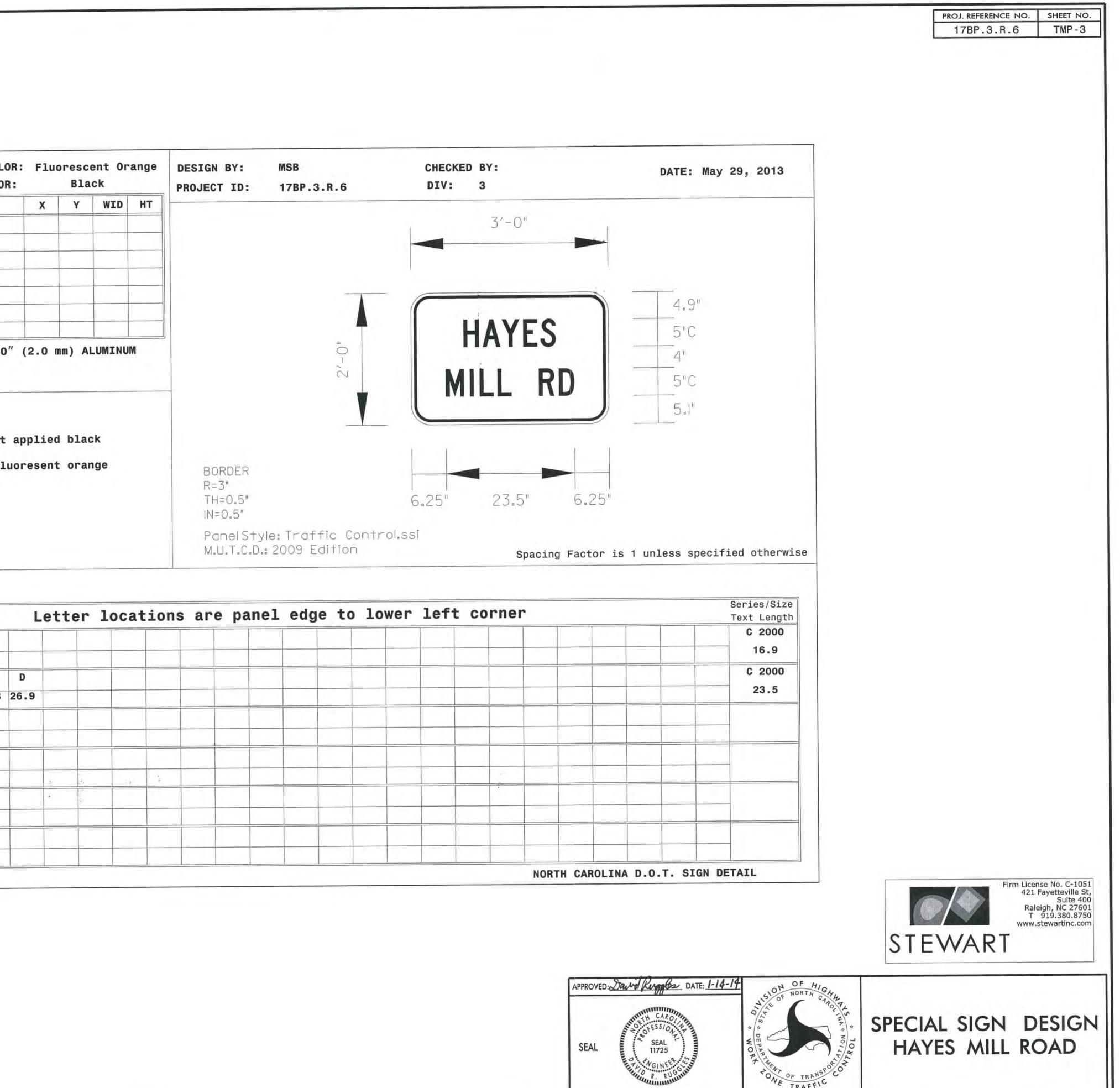




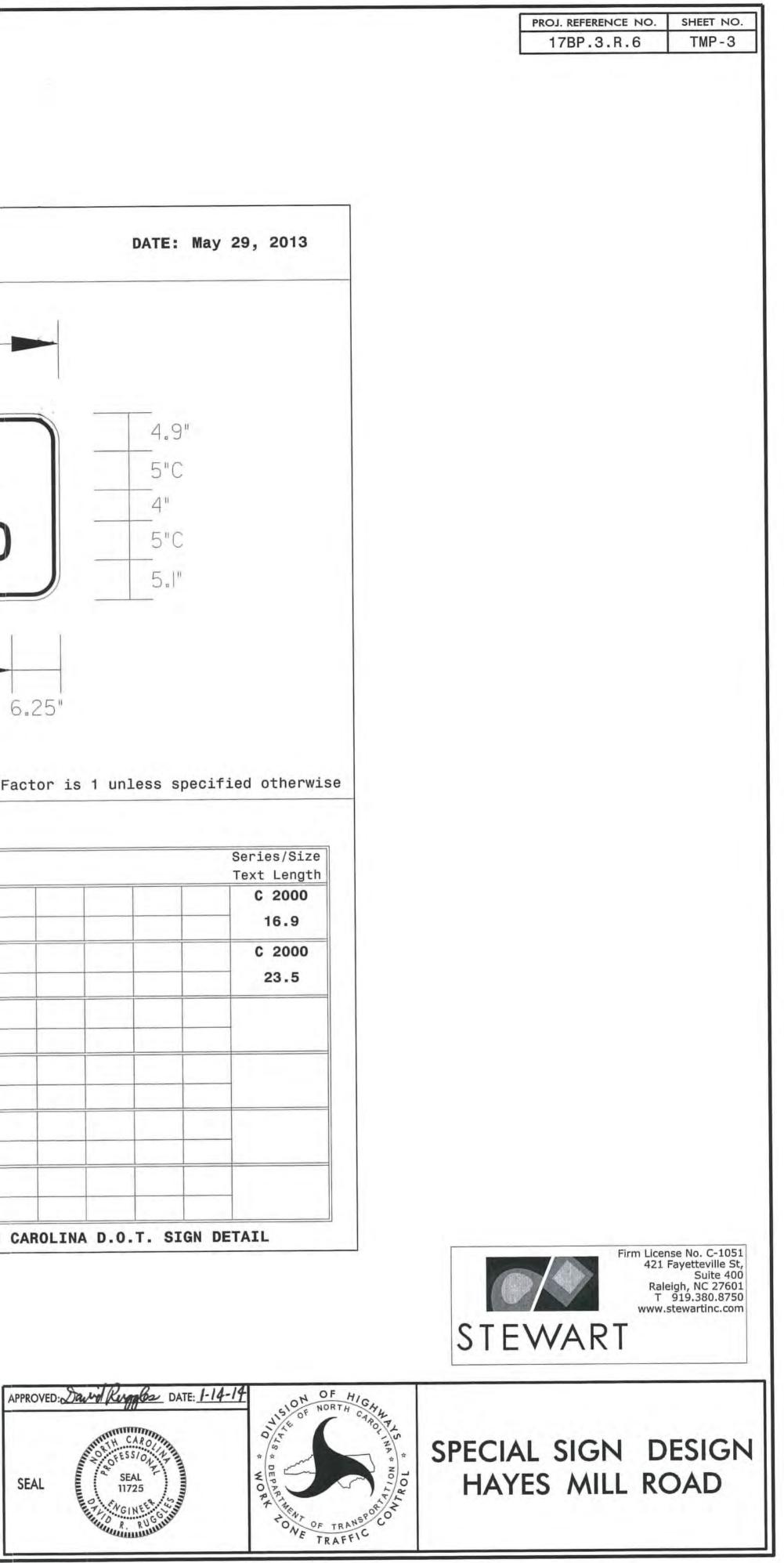


SIGN	NUMBER	: TMP	-3		BACK	
	TYPE	: STA	TIONA	RY	COPY	COL
QU	ANTITY	: 6		5	SYMBOL	
SIGN	WIDTH	: 3'-0	0″			
	HEIGHT					
TOTAL A	AREA:	6.0	Sq.F	't		
	R TYPE			-		
	RECESS			-		
	RADII		,			
NO	Z BARS			M	AT'L:	0.08
	LENGTH					
2. Ba	ckgrou	nu sn lecti		e NC	GRADE	B 1
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re	TER PO	lecti SITIC	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R
re	TER PO	A 13.1	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R
re	TER PO	A 13.1 I	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R
re	TER PO	A 13.1 I	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R
re	TER PO	A 13.1 I	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R
re	TER PO	A 13.1 I	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R
re	TER PO	A 13.1 I	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R
re	TER PO	A 13.1 I	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R
re	TER PO	A 13.1 I	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R
re	TER PO	A 13.1 I	Ve sh DNS 16.6 L	E 20.5 L	Ig. S 23.6	R

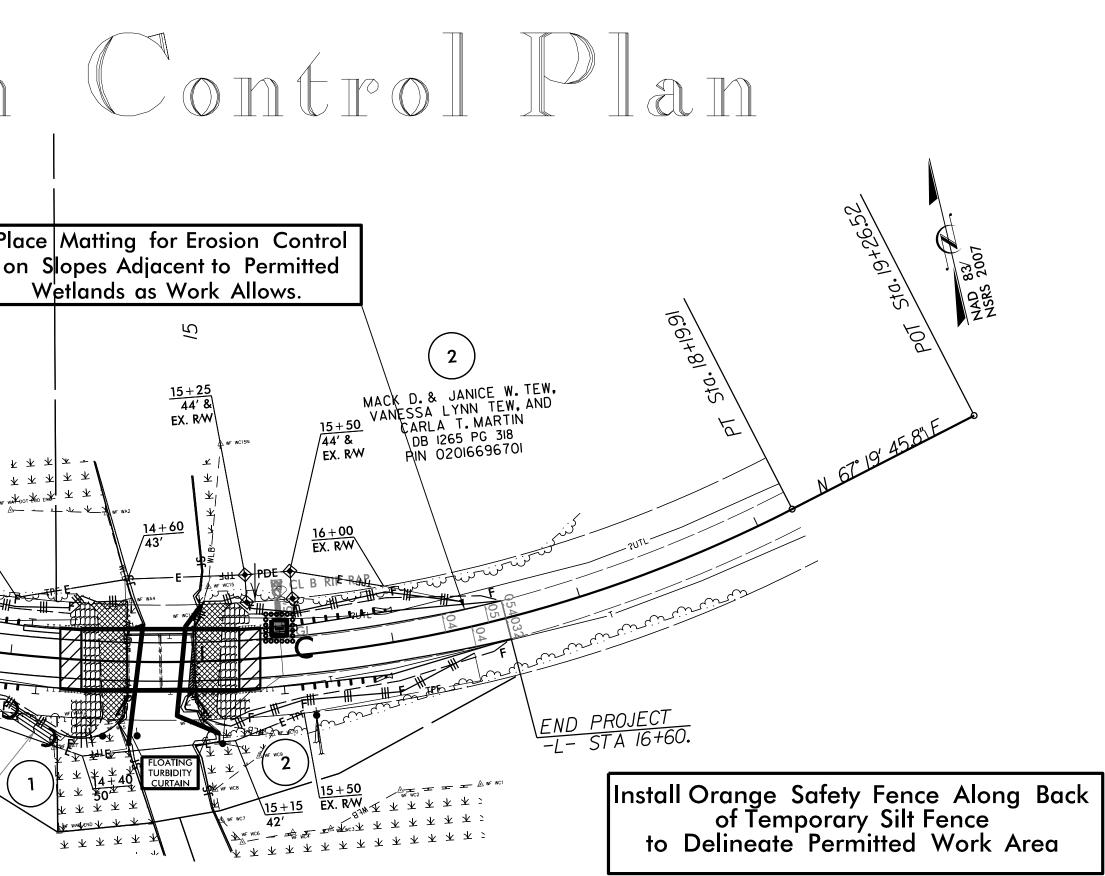
-3

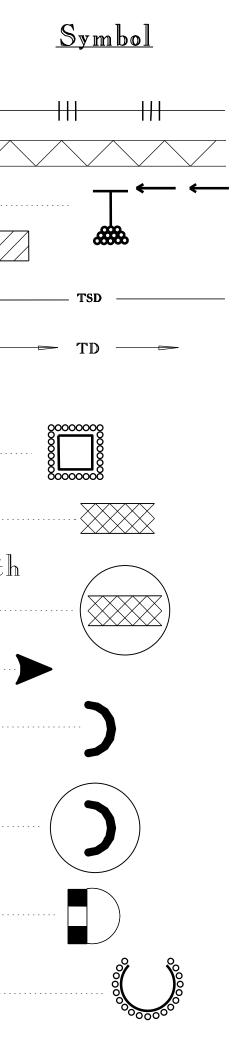


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201-00-00-00-00-00-00-00-00-00-00-00-00-0	Pla or SHELBY T.& SAMUEL H. JACKSON / CONNIE S. & TERRY F. GAUSE DB 1265 PG 320 PIN 02100380002
	ROBERT M. BAREFOOT         REVOCABLE TRUST         DB 1289 PG 110         PIN 02076832002         BEGIN PROJECT         -L- STA 12+50.00         RR SPIKE IN         UTILITY POLE         ELEV=129.00
Std.#	Description
<u>Std.</u> 1605.01 1606.01	Description Temporary Silt Fence
<u>Std.</u> 1605.01	Temporary Silt Fence
<u>Std.</u> 1605.01 1606.01	Temporary Silt Fence
<u>Std.</u> 1605.01 1606.01 1622.01	Temporary Silt Fence
<u>Std.</u> 1605.01 1606.01 1622.01 1630.02	Temporary Silt Fence — III- Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B
<u>Std.</u> 1605.01 1606.01 1622.01 1630.02 1630.03	Temporary Silt Fence — — — — — — — — — — — — — — — — — — —
<u>Std.</u> 1605.01 1606.01 1622.01 1630.02 1630.03 1630.05	Temporary Silt Fence Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B Temporary Silt Ditch Temporary Diversion
<u>Std.</u> 1605.01 1606.01 1622.01 1630.02 1630.03 1630.05 1630.06	Temporary Silt Fence Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B Temporary Silt Ditch Temporary Diversion Special Stilling Basin
<u>Std.</u> 1605.01 1606.01 1622.01 1630.02 1630.03 1630.05 1632.03	Temporary Silt Fence Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B Temporary Silt Ditch Temporary Diversion Special Stilling Basin Rock Inlet Sediment Trap Type C
<u>Std.</u> 1605.01 1606.01 1622.01 1630.02 1630.03 1630.05 1632.03	Temporary Silt Fence Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B Temporary Silt Ditch Temporary Diversion Special Stilling Basin Rock Inlet Sediment Trap Type C Temporary Rock Silt Check Type-A Temporary Rock Silt Check Type-A
<u>Std.</u> 1605.01 1606.01 1622.01 1630.02 1630.03 1630.05 1632.03 1633.01	Temporary Silt Fence Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B Temporary Silt Ditch Temporary Diversion Special Stilling Basin Rock Inlet Sediment Trap Type C Temporary Rock Silt Check Type-A Temporary Rock Silt Check Type-A Matting and Polyacrylamide (PAM)
<u>Std.</u> 1605.01 1606.01 1622.01 1630.02 1630.03 1630.05 1632.03 1633.01	Temporary Silt Fence Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B Temporary Silt Ditch Temporary Diversion Special Stilling Basin Rock Inlet Sediment Trap Type C Temporary Rock Silt Check Type=A Temporary Rock Silt Check Type=A Matting and Polyacrylamide (PAM) Temporary Rock Silt Check Type=B
<u>Std.</u> 1605.01 1606.01 1622.01 1630.02 1630.03 1630.05 1632.03 1633.01	Temporary Silt Fence Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B Temporary Silt Ditch Temporary Diversion Special Stilling Basin Rock Inlet Sediment Trap Type C Temporary Rock Silt Check Type=A Temporary Rock Silt Check Type=A with Matting and Polyacrylamide (PAM) Temporary Rock Silt Check Type=B Wattle





Hang Orange Flagging Along Wattle Barrier Under Bridge to Delineate Permitted Work Area

50	25	0	50

100

 PROJECT REFERENCE NO.
 SHEET NO.

 17BP.3.R.6
 EC-01/CONST.04

 RW SHEET NO.
 RW SHEET NO.

 ROADWAY DESIGN ENGINEER
 HYDRAULICS ENGINEER

Jenny Fleming, PE LEVEL IIIA NAME

3340

LEVEL III CERTIFICATION NO.

## ROADSIDE ENVIRONMENTAL UNIT DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS RALEIGH, N.C.

2012 STANDARD SPECIFICATIONS

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 3, 2011 ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER.

> ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER.

## 2012 STANDARD DRAWINGS

1604.01	Railroad Erosion Control Detail	1632.01	Rock Inlet Sediment Trap Type A
1605.01	Temporary Silt Fence	1632.02	Rock Inlet Sediment Trap Type B
1606.01	Special Sediment Control Fence	1632.03	Rock Inlet Sediment Trap Type C
1607.01	Gravel Construction Entrance	1633.01	Temporary Rock Silt Check Type A
1622.01	Temporary Berms and Slope Drains	1633.02	Temporary Rock Silt Check Type B
1630.01	Riser Basin	1634.01	
1630.02	Silt Basin Type B	1634.02	
1630.03	Temporary Silt Ditch	1635.01	
1630.04	Stilling Basin	1635.02	Rock Pipe Inlet Sediment Trap Type B
1630.05	Temporary Diversion	1640.01	Coir Fiber Baffle
1630.06	- F	1645.01	Temporary Stream Crossing
1631.01	Matting Installation		

# SOIL

# SITE DESCRIPTION

PERIMETER DIKES, SWALES, DITCHES AND

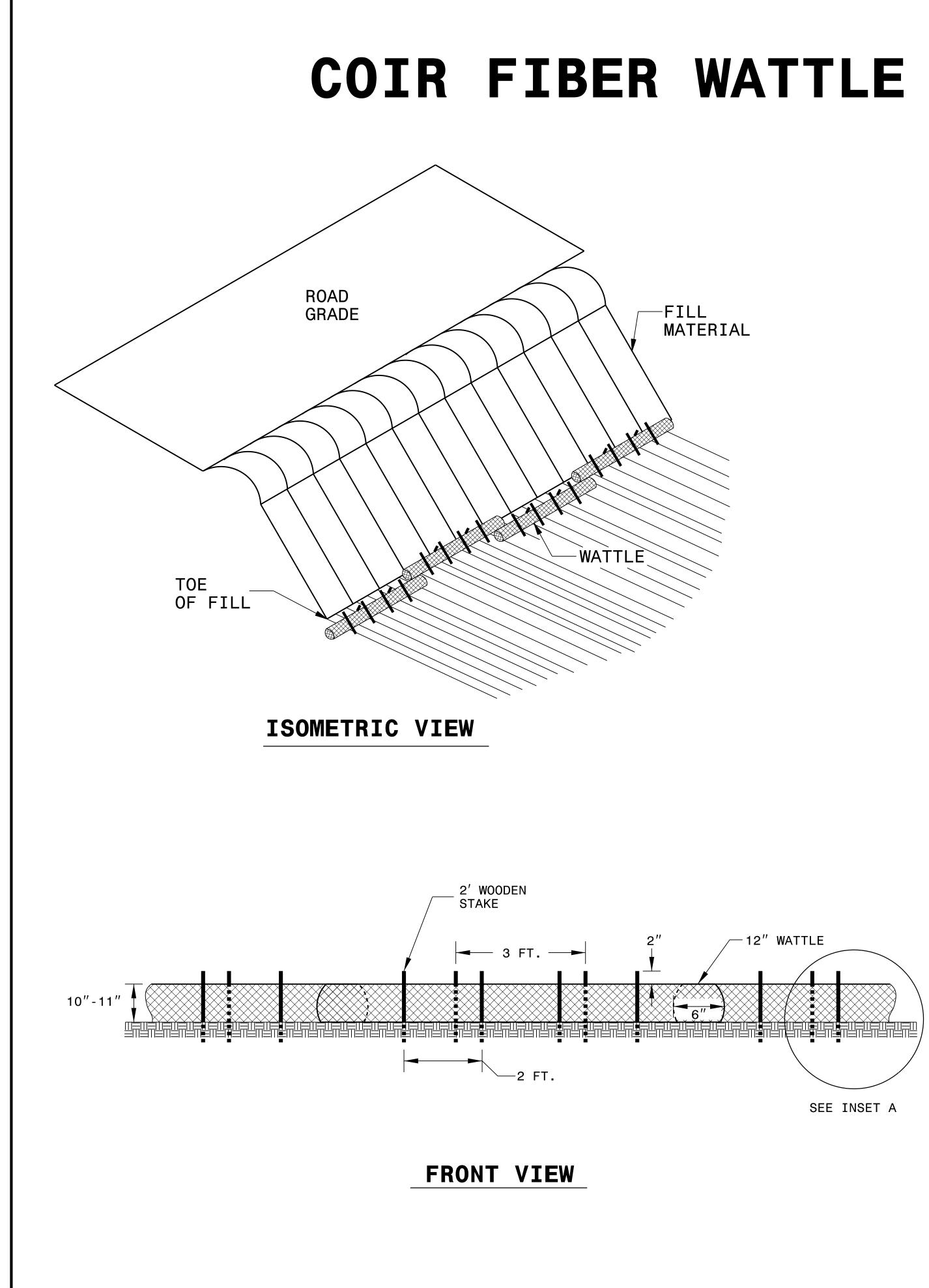
HIGH QUALITY WATER (HQW) ZONES

SLOPES STEEPER THAN 3:1

SLOPES 3:1 OR FLATTER

ALL OTHER AREAS WITH SLOPES FLATTER

	VISION OF HIGHW Fe of North Ca		PROJECT REFERENCE NO. SHEET 17BP.3.R.6 EC-C
<b>STABI</b>	LIZATION	TIM	EFRAMES
	STABILIZATION	TIME	TIMEFRAME EXCEPTIONS
SLOPES	7 DAYS		NONE
	7 DAYS		NONE
	7 DAYS		IF SLOPES ARE IO'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1,14 DAYS ARE ALLOWED.
	14 DAYS		7 DAYS FOR SLOPES GREATER THAN 50' IN LENGTH.



# **COIR FIBER WATTLE BARRIER DETAIL**

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

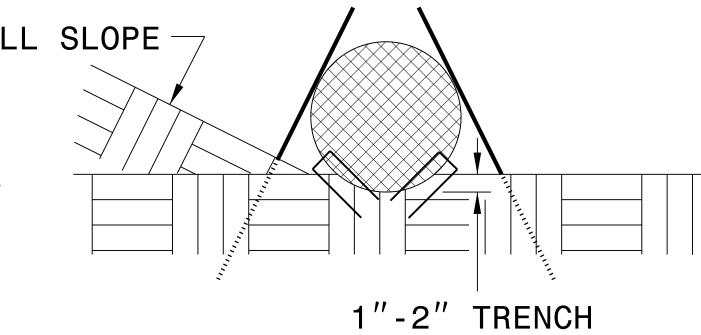
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

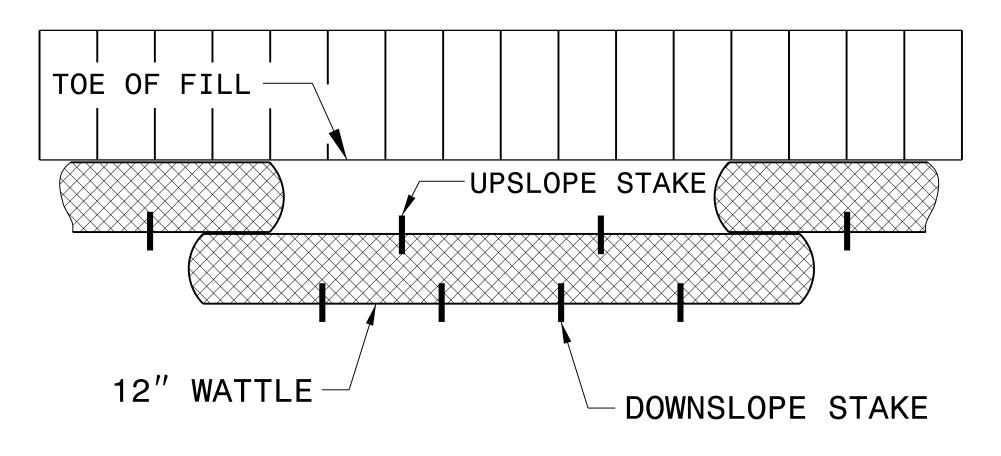
INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 20 FT.

# FILL SLOPE

INSET A

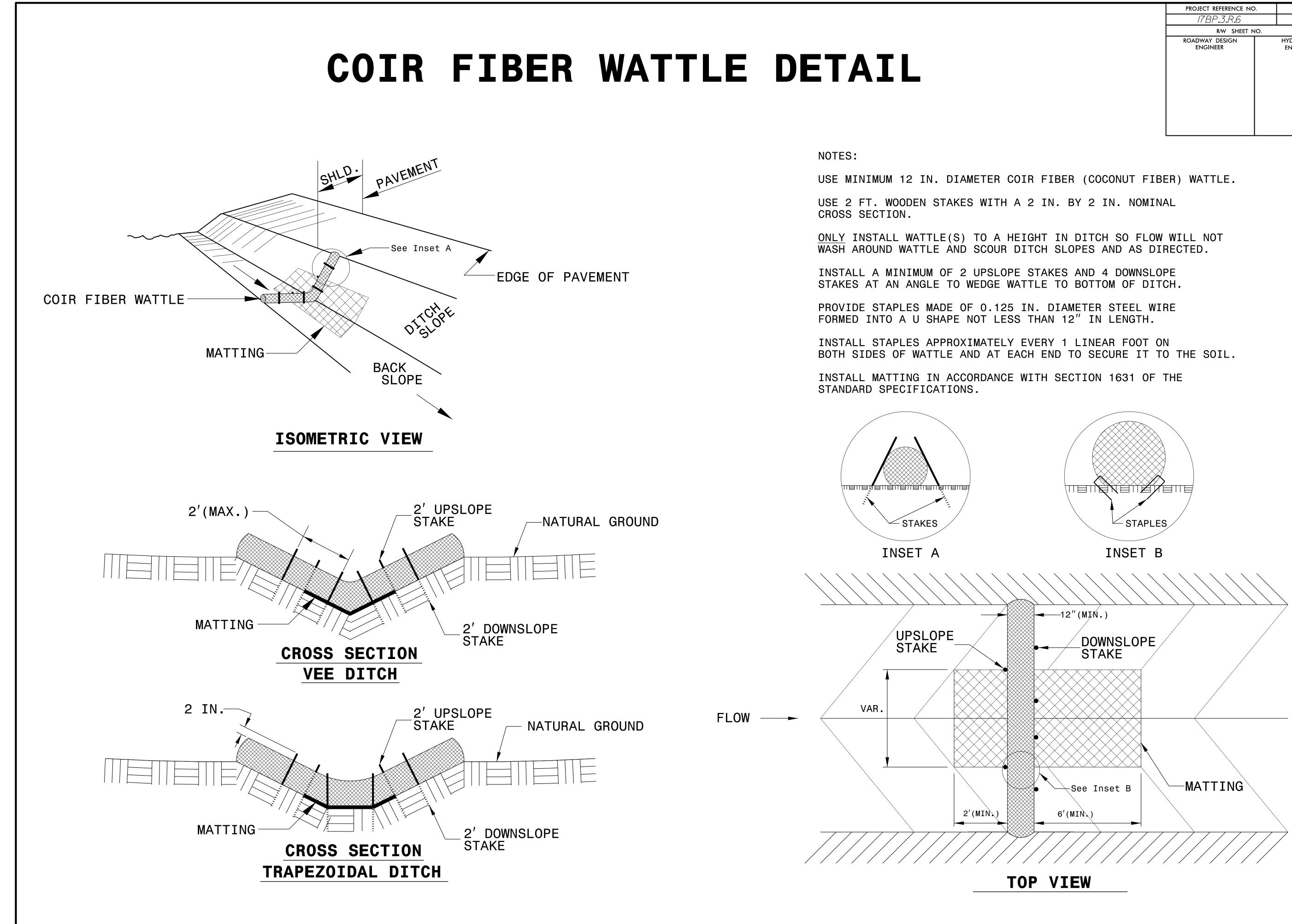




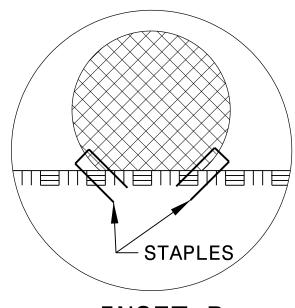
PROJECT REFERENCE NO	. SHEET NO.
17BP.3.R.6	EC-03
R/W SHEET N	Ю.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL

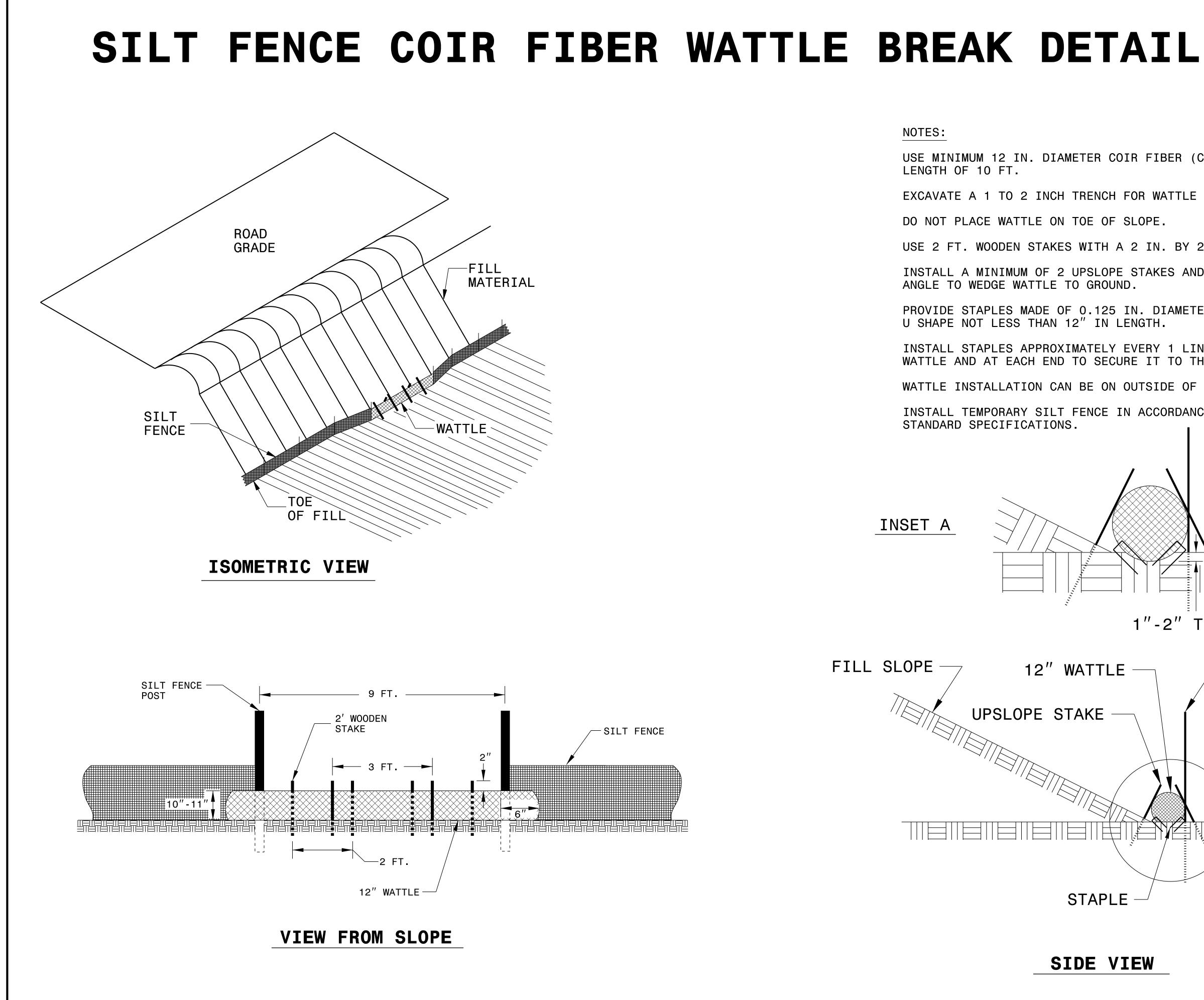
TOP VIEW



PROJECT REFERENCE NC	SHEET NO.				
17BP.3.R.6	17BP.3.R.6 RW SHEET NO.				
R/W SHEET N					
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER			







NOTES:

LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

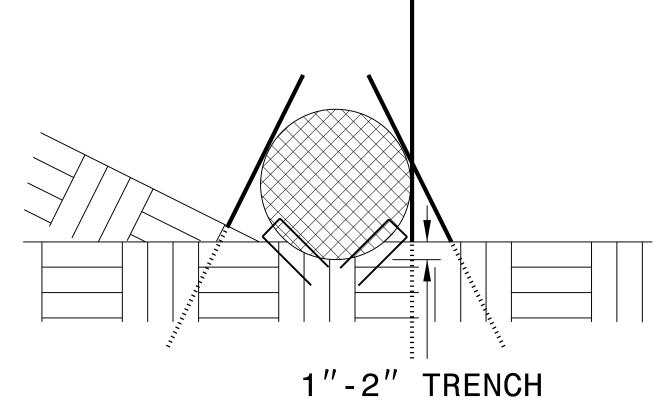
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

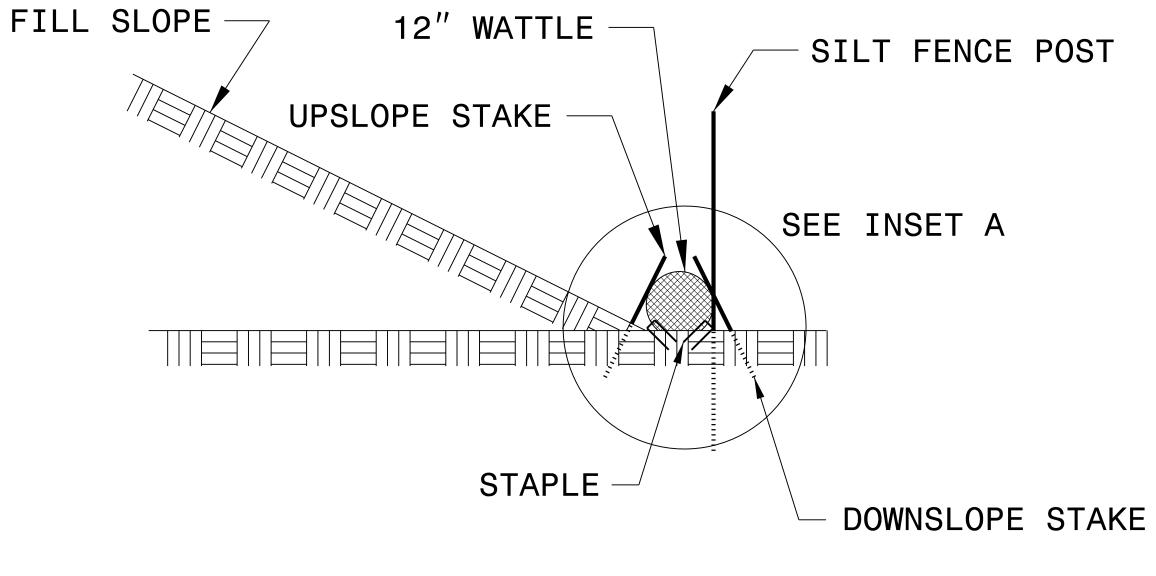
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

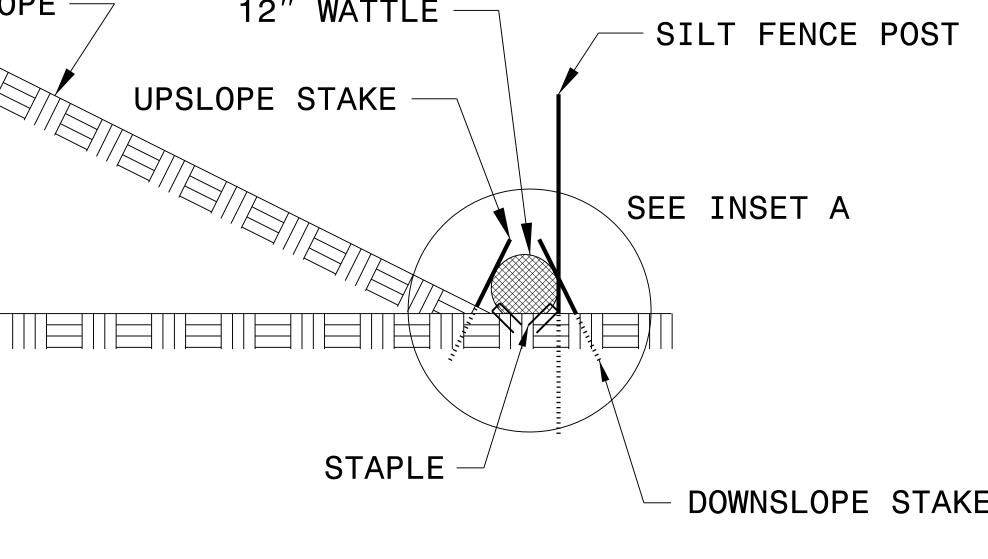
INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED. INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.

INSET A



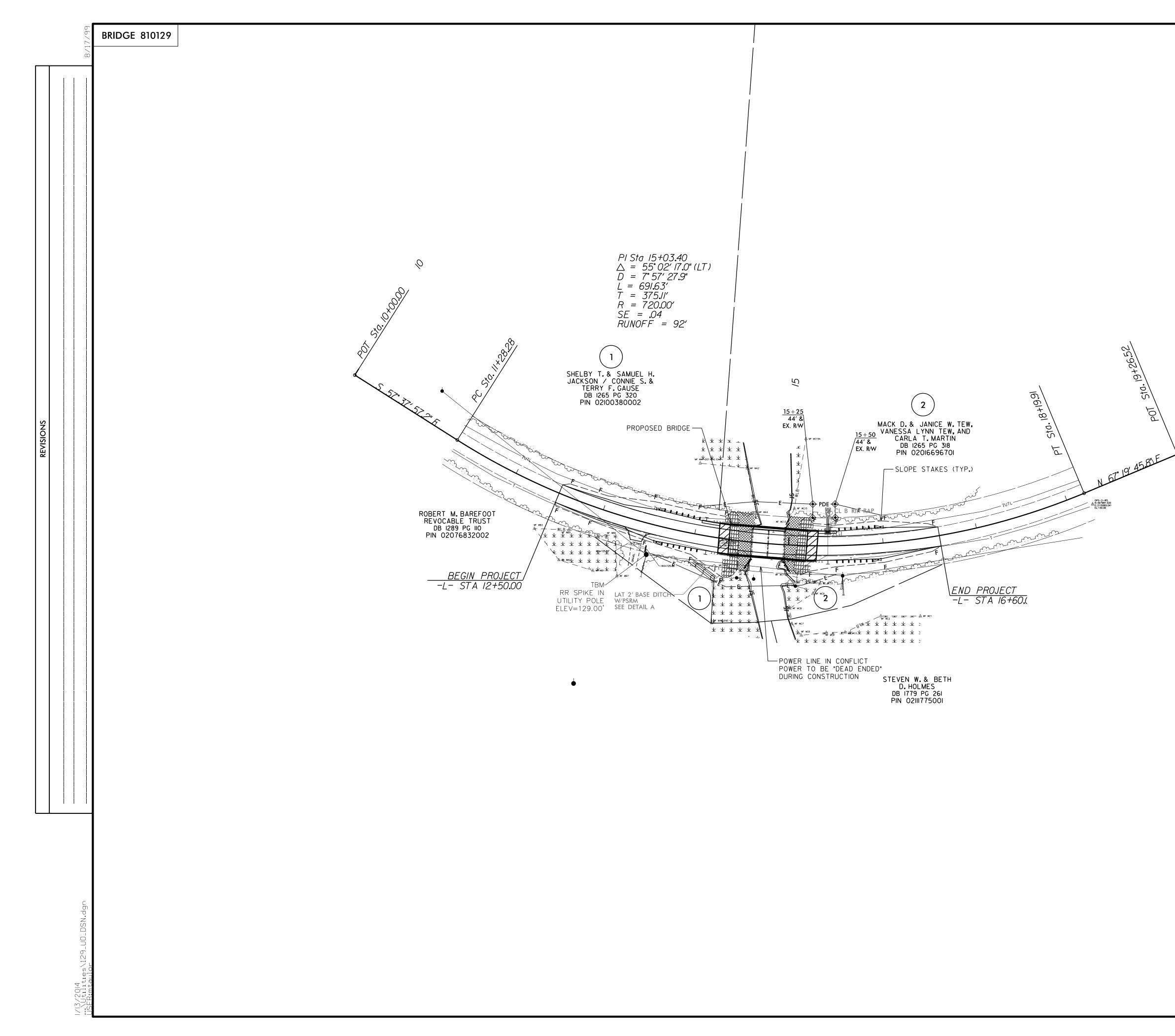


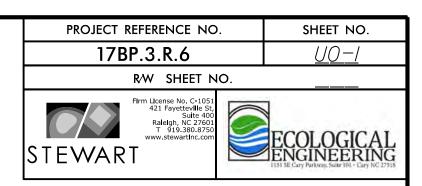


PROJECT REFERENCE NC	D. SHEET NO.
17BP.3.R.6	EC-05
R/W SHEET N	10.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

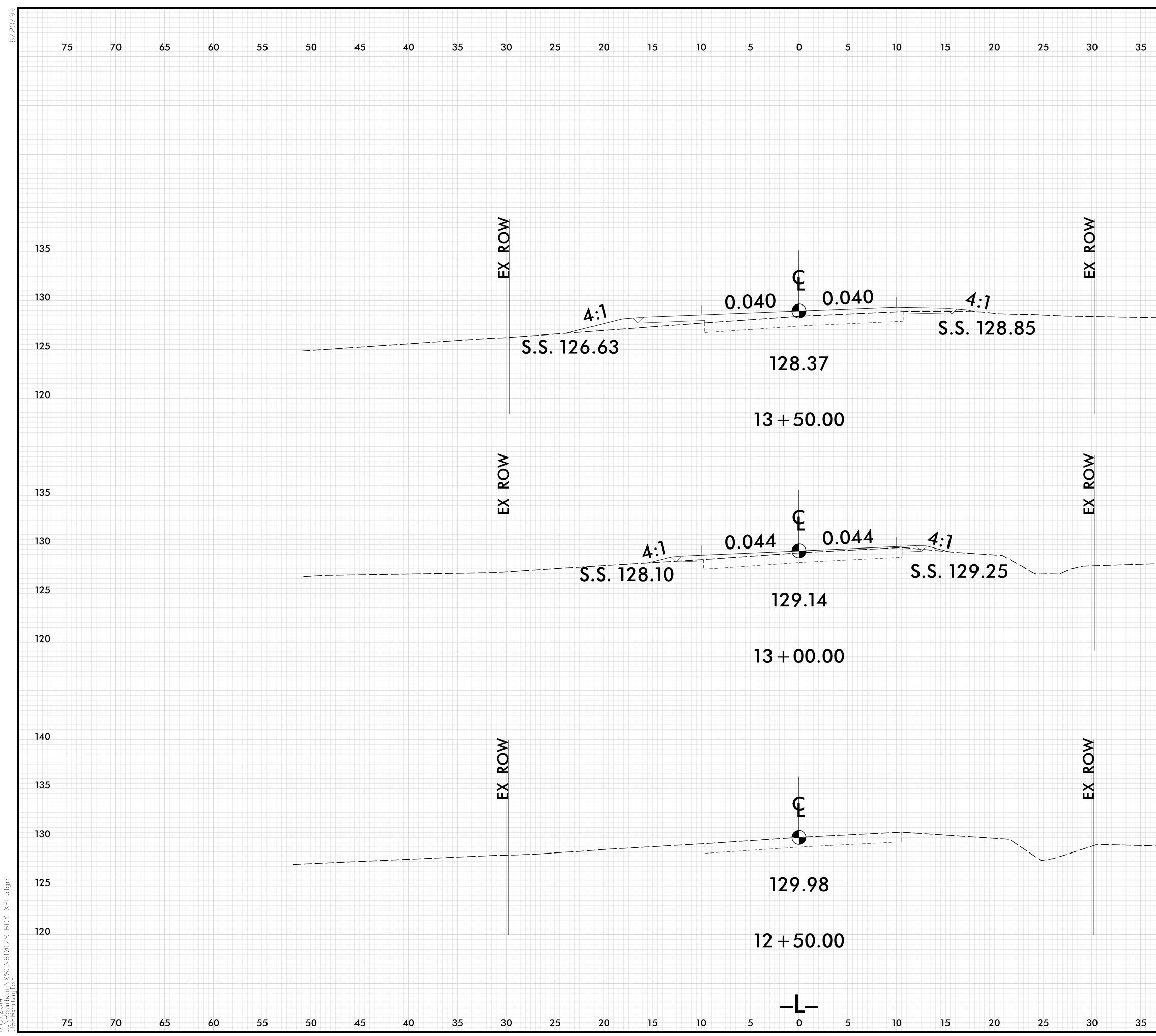
USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND

SIDE VIEW



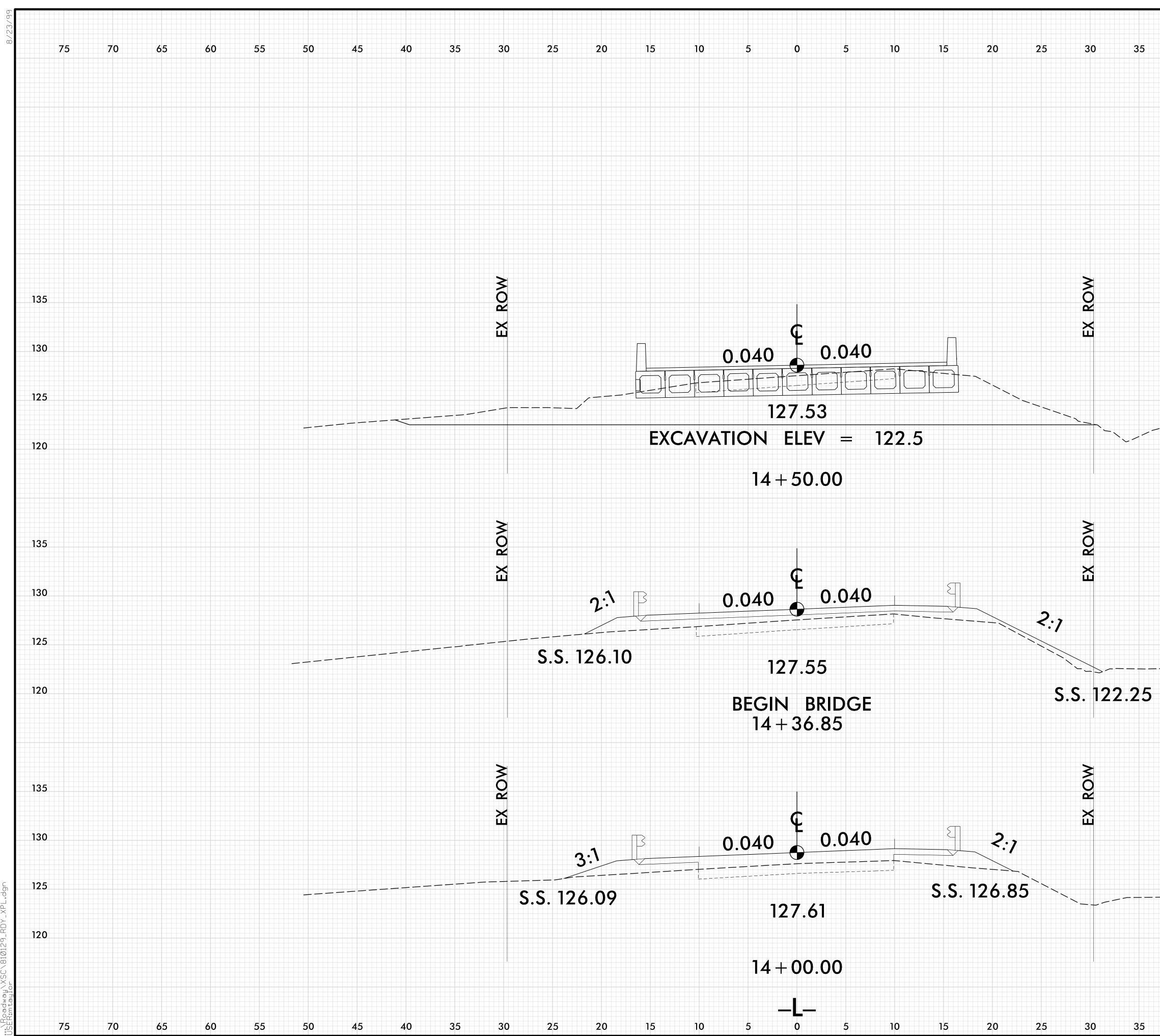






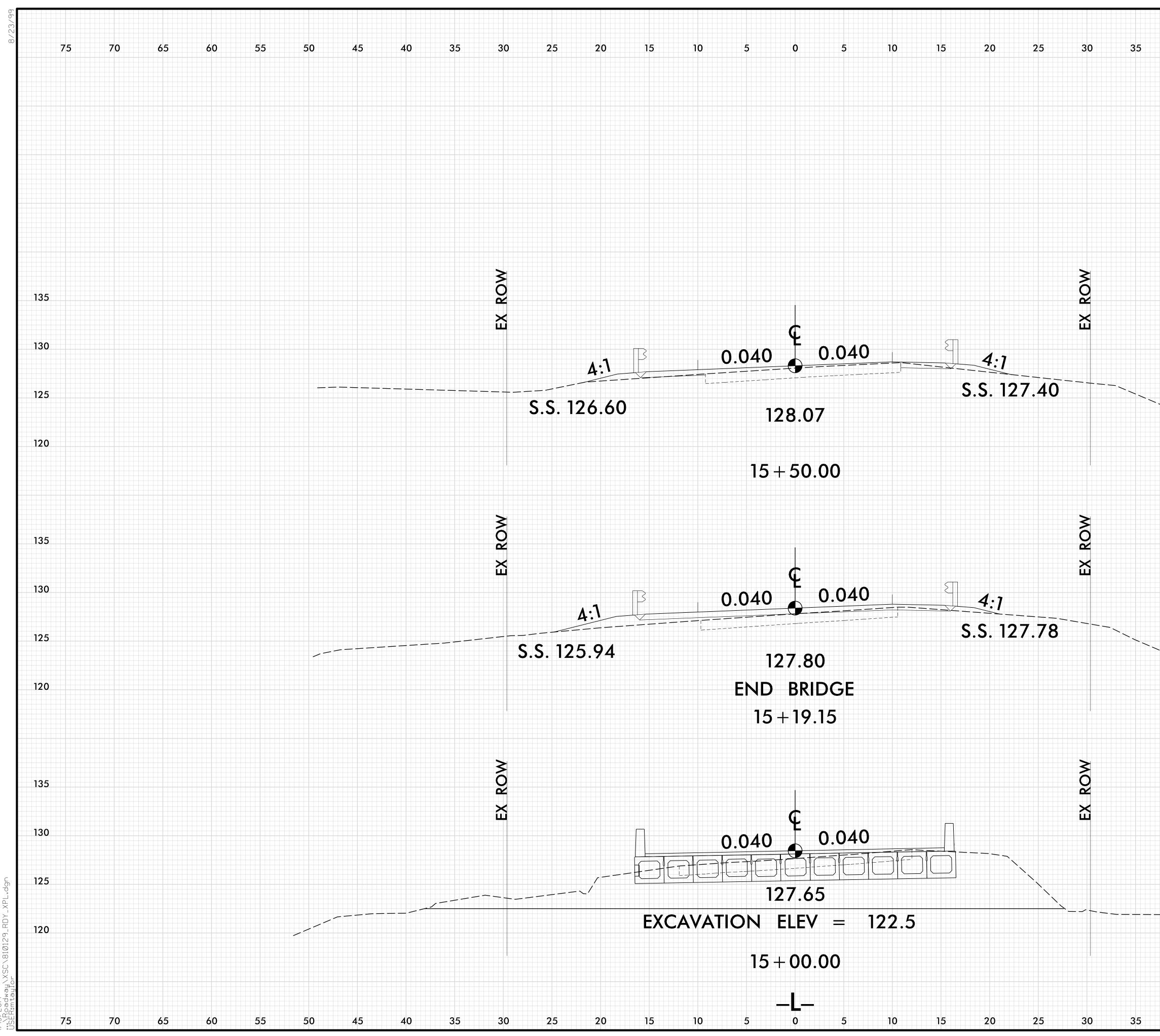
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		0	2.5 5	PRO	OJ. REFEREN 17BP.3.R		sheet no. X–1
40	45	50	55	60	65	70	75
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	/						130
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							120
40	45	50	55	60	65	70	75

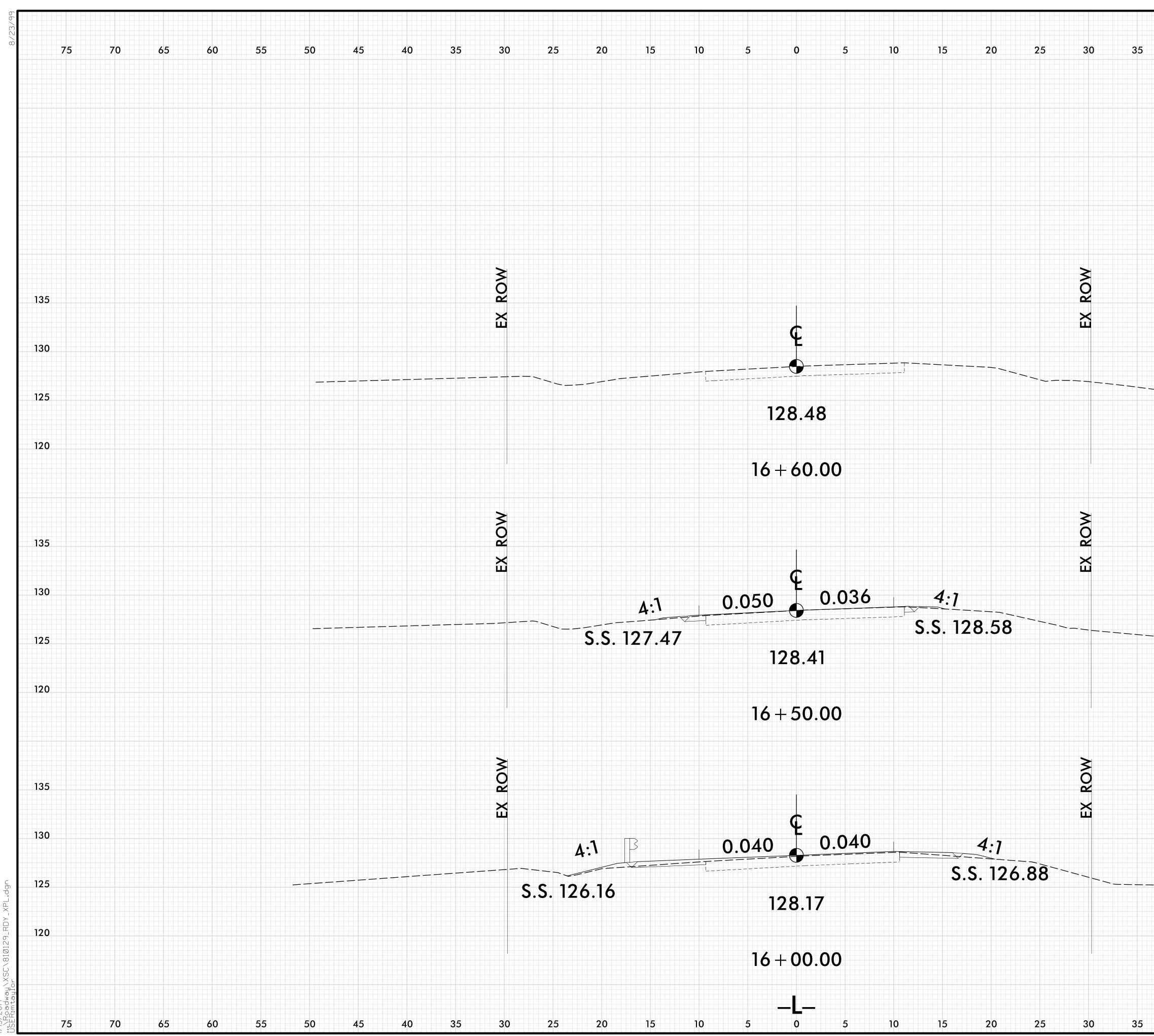


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		<b>2</b> .5 5	PRC	PROJ. REFERENCE NO. 17BP.3.R.6			
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40 45	50	55	60	65	70	75	

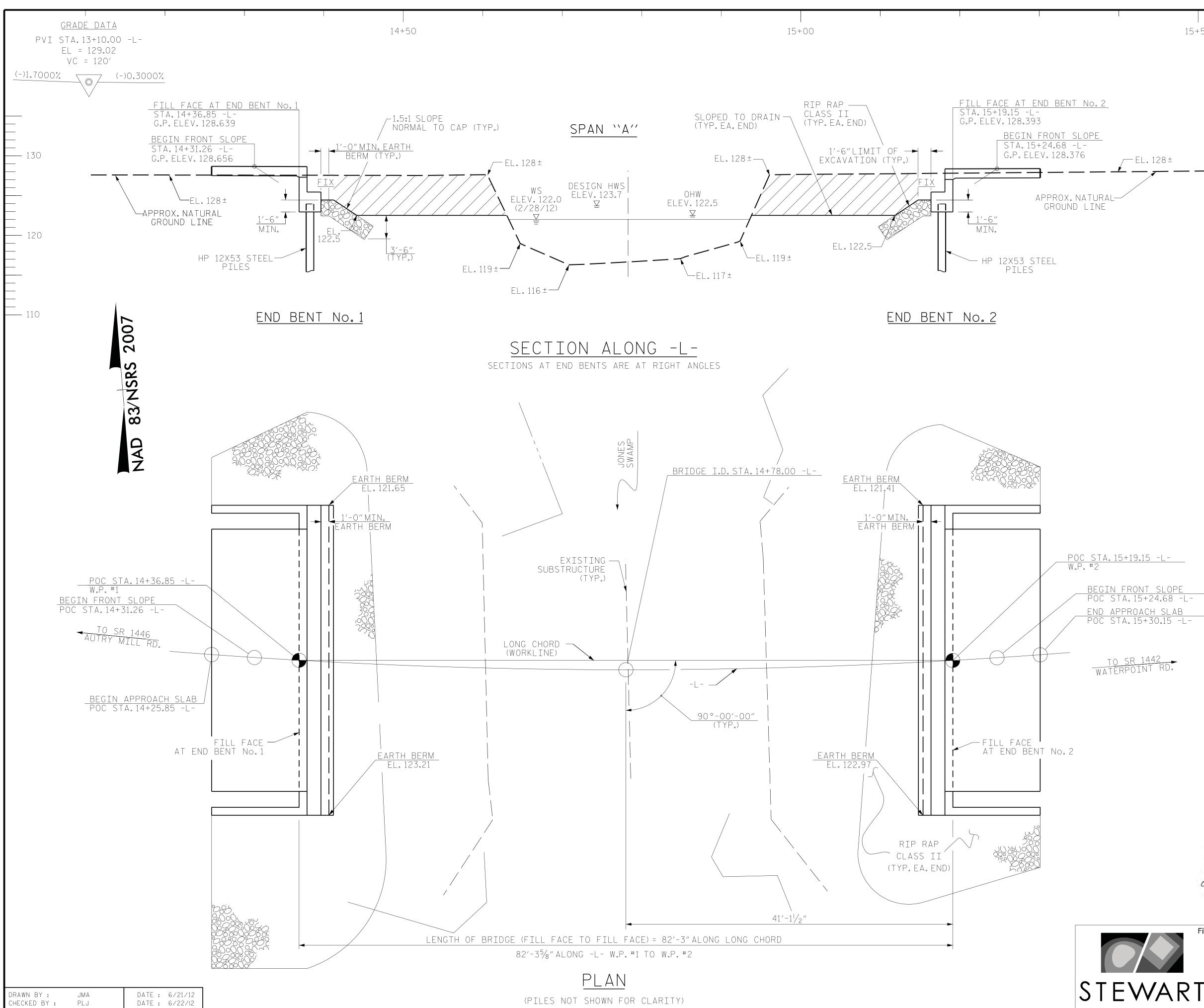


		C	) 2.5 5	P	ROJ. REFEREN 17BP.3.R		SHEET NO. X–3		
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40	45	50	55	60	65	70	75		



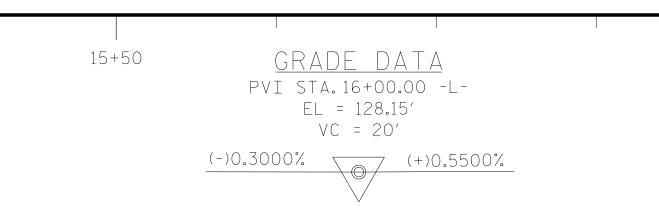
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		2.5 5	PR	OJ. REFEREN 17BP.3.R		SHEET NO. X–4		
40 45	50	55	60	65	70	75		
						135		
						130		
						125		
						120		
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						125		
						120		
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						130		
=						125		
						120		



<u>PROACH SLAB</u> A.15+30.15 -L-		I HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS
<u>SR 1442</u> RPOINT RD.		
		PROJECT NO. <u>17BP.3.R.6</u>
		SAMPSON COUNTY
		STATION: 14+78.00 -L-
		SHEET 1 OF 3 REPLACES BRIDGE No. 129
	OFESSION A	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
	Taynel 142	GENERAL DRAWING
	Z-28-13	FOR BRIDGE OVER Jones Swamp on SR 1445
	Firm License No. C-1051 421 Fayetteville St, Suite 400 Raleigh, NC 27601 T 919.380.8750	BETWEEN SR 1446 AND SR 1442
	www.stewartinc.com	REVISIONS SHEET NO.
WAR	Т	NO.BY:DATE:DATE:DATE:DATE:13TOTAL SHEETS2416

HORIZONTAL CURVE DATA FOR -L-PI STA.15+03.40 △ = 55° 02′17.0″(LT) D = 7° 57′27.9″ L = 691.63′ T = 375.11 R = 720.00′

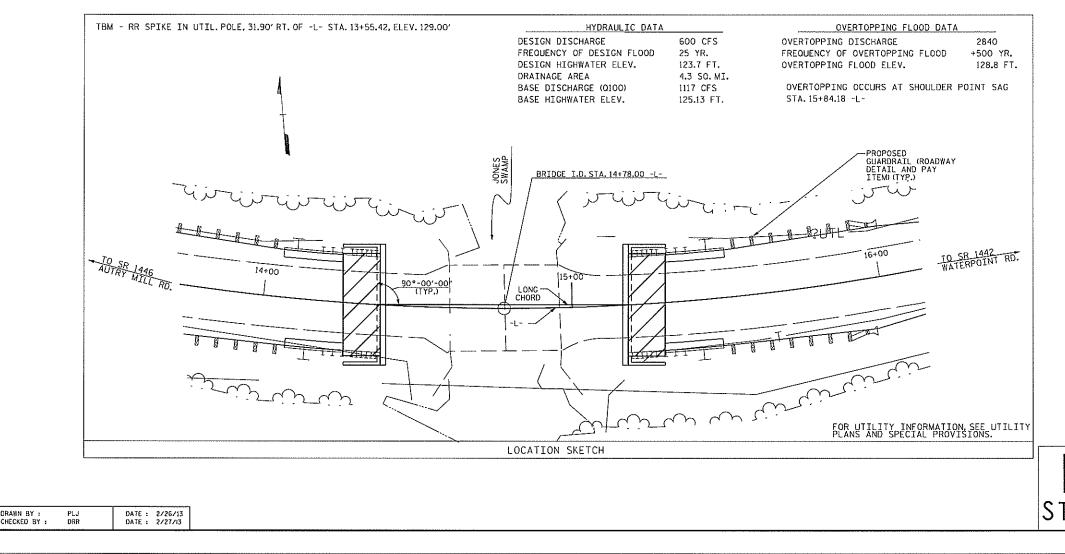


### GENERAL NOTES:

- 1. ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
- 2. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY 2001.
- 4. REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.
- 5. THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20 FT.EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER.SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.
- 6. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- 7. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

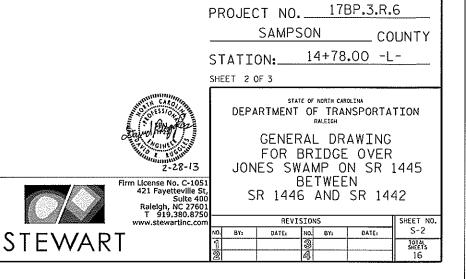
- 7. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- 8. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- 9. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- 10. FOR ALL OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- 11. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- 12. THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
- 13. ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

	REMOVAL OF EXISTING STRUCTURE AT STATION 14+78.00 -L-	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION AT STATION 14+78.00 -L-	CLASS A CONCRETE	BRIDGE APPROACH SLABS AT STATION 14+78.00 -L-	REINFORCING STEEL	н	IP 12X53 STEEL PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	ELASTOMERIC BEARINGS	PRE CON	O″× 2'-9" ESTRESSEC CRETE BO BEAMS
	LUMP SUM	EACH	LUMP SUM	CY	LUMP SUM	LBS	No.	LF	EACH	LF	TON	LUMP SUM	No.	LF
SUPERSTRUCTURE	LUMP SUM	<u></u>			LUMP SUM		-			160.0		LUMP SUM	11	880
END BENT No.1		1	LUMP SUM	17.0		2812	7	315	3		110		-	
END BENT No. 2			LUMP SUM	17.0		2812	7	315	3		90			
TOTAL	LUMP SUM	1	LUMP SUM	34.0	LUMP SUM	5624	14	630	6	160.0	200	LUMP SUM	11	880

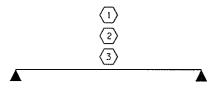


### FOUNDATION NOTES:

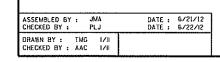
- 1. PILES AT END BENT No.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 75 TONS PER PILE.
- 2. DRIVE PILES AT END BENT No.1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 125 TONS PER PILE.
- 3. TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED AT END BENTS No.1 OR 2. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- 4. PILE RESTRIKES ARE RECOMMENDED.



		LOAD AN	ID RE	SIST	FANCE	E FA(	CTOR	RAT	ING	(LRF	FD) S	UMMA	ry f	OR	PRES	TRES	SSED	CON	CRET	E GI	RDEF	RS		
										STRE	ENGTH	I LIN	AIT ST	TATE				SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			-
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.155		1.75	0.273	1.72	A	EL	39.25	0.502	1.51	A	EL	7.85	0,80	0.273	1.15	A	EL	39.25	
DESIGN		HL-93(0pr)	N/A		1.958		1.35	0.273	2.23	А	EL	39.25	0.502	1.96	А	EL	7.85	NZA						
LOAD RATING	ſ	HS-20(Inv)	36.000	2	1.533	55.181	1.75	0.273	2.28	А	EL	39.25	0.502	1.91	А	EL	7.85	0.80	0.273	1.53	А	EL	39.25	
		HS-20(0pr)	36.000		2.473	89.021	1.35	0.273	2.96	А	EL	39.25	0.502	2.47	A	EL	7.85	N/A						
		SNSH	13.500		3.509	47.376	1.4	0.273	6.53	A	EL	39.25	0,502	5.73	А	EL	7.85	0.80	0.273	3.51	A	EL	39.25	
	[	SNGARB52	20.000		2.594	51.88	1.4	0.273	4.82	А	EŁ	39.25	0.502	4.06	А	EL	7.85	0.80	0.273	2.59	A	EL	39.25	
		SNAGRIS2	22.000		2.448	53.85	1.4	0.273	4.55	A	EL	39.25	0.502	3.76	A	EL	7.85	0.80	0.273	2.45	A	EL	39.25	
		SNCOTTS3	27.250		1.746	47.571	1.4	0.273	3.25	A	EL	39.25	0.502	2.86	А	EL	7.85	0.80	0.273	1.75	A	EL	39.25	
	l s	SNAGGRS4	34.925		1.451	50.667	1.4	0.273	2.7	А	EL	39.25	0.502	2,36	A	EL	7.85	0.80	0.273	1.45	A	EL	39.25	
		SNS5A	35.550		1.419	50.453	1.4	0.273	2.64	А	EL	39.25	0.502	2.38	A	EL	7.85	0.80	0.273	1.42	A	EL	39.25	
		SNS6A	39.950		1.299	51.885	1.4	0.273	2,42	A	EL	39.25	0.502	2.17	A	EL	7.85	0.80	0.273	1.30	A	EL	39.25	
LEGAL		SNS7B	42.000		1.237	51.941	1.4	0.273	2.3	А	EL	39.25	0.502	2.13	А	EL	7.85	0.80	0.273	1.24	A	EL	39.25	
LOAD		TNAGRIT3	33.000		1.583	52.231	1.4	0.273	2.94	A	EL	39.25	0,502	2.59	А	EL	7.85	0.80	0.273	1.58	Α	EL	39.25	1
RATING		TNT4A	33.075		1.589	52.55	1.4	0.273	2.96	A	EL	39.25	0.502	2.53	A	EL	7.85	0.80	0.273	1.59	A	EL	39.25	-
		TNT6A	41.600		1.296	53.907	1.4	0.273	2.41	A	EL	39.25	0.502	2.25	А	EL	7.85	0.80	0.273	1.30	A	EL	39.25	
	5	TNT7A	42.000		1.301	54.625	1.4	0.273	2.42	A	EL	39.25	0.502	2.21	А	EL	7.85	0.80	0.273	1.30	A	εl	39.25	-
		TNT7B	42.000		1.341	56.333	1.4	0.273	2.49	A	EL	39.25	0.502	2.08	A	EL	7.85	0.80	0.273	1.34	A	EL	39.25	1
		TNAGRIT4	43.000		1.279	55.001	1.4	0.273	2.38	A	EL	39.25	0.502	2.02	A	EL	7.85	0.80	0.273	1.28	A	EL	39.25	1
	İ İ	TNAGT5A	45.000		1.207	54.337	1.4	0.273	2.25	Δ	£L	39.25	0.502	2	A	EL	7.85	0.80	0.273	1.21	A	EL	39.25	1
		TNACT5B	45.000	3	1.194	53.739	1.4	0.273	2.22	Α	EL	39.25	0.502	1.92	A	EL	7.85	0.80	0.273	1.19	A	EL	39.25	1



LRFR SUMMARY





### LOAD FACTORS:

DESIGN	LIMIT STATE	γ _{DC}	γ _{DW}
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

### NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REOUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

COMMENTS:

- 1. 2.
- 3.

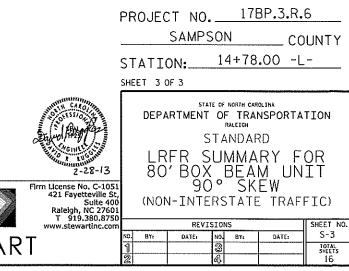
- 4.



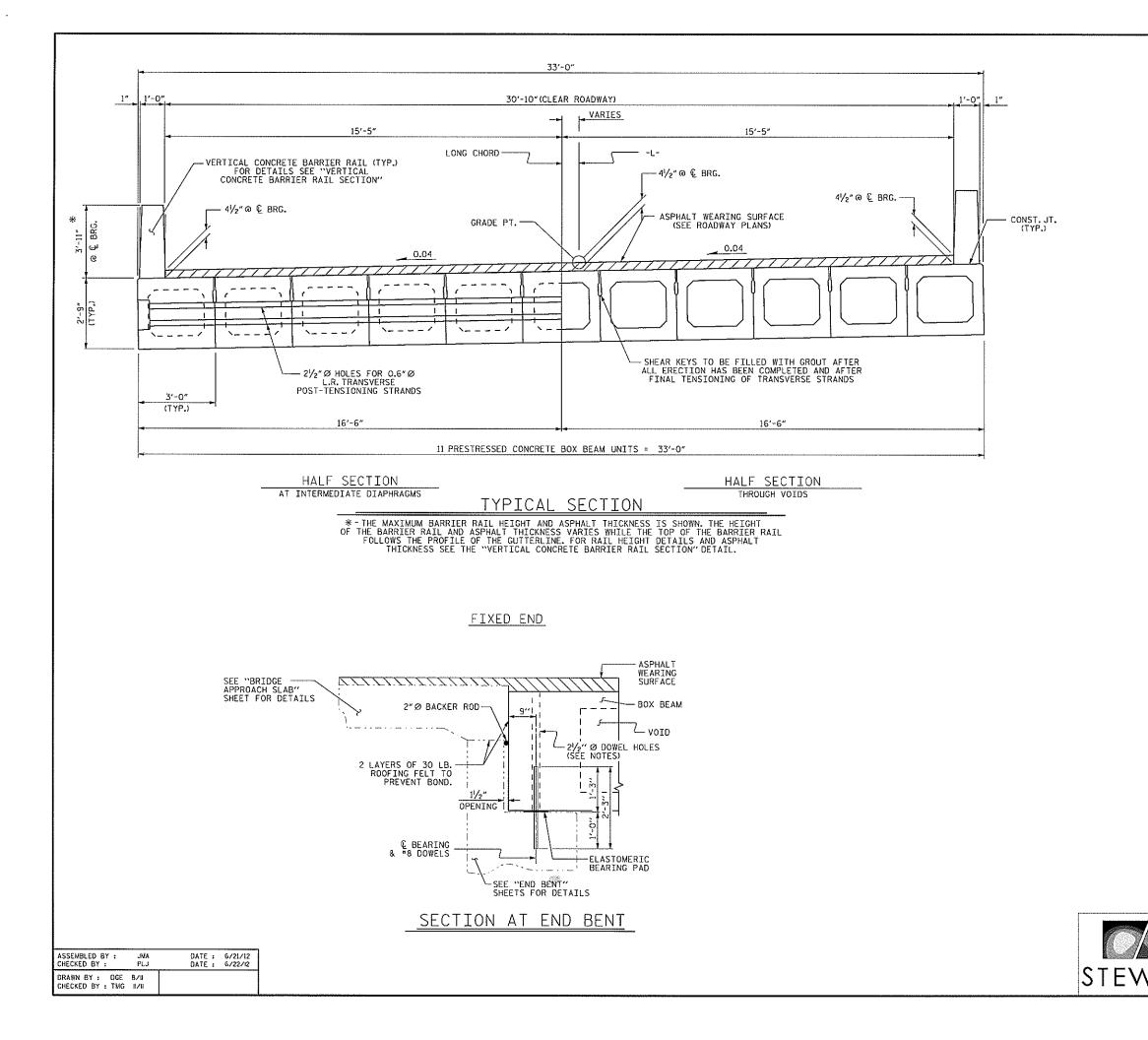


2 DESIGN LOAD RATING (HS-20) (3) LEGAL LOAD RATING ** ** SEE CHART FOR VEHICLE TYPE GIRDER LOCATION I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER



STD. NO. 33LRFR1_90S_80L



### NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6000 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

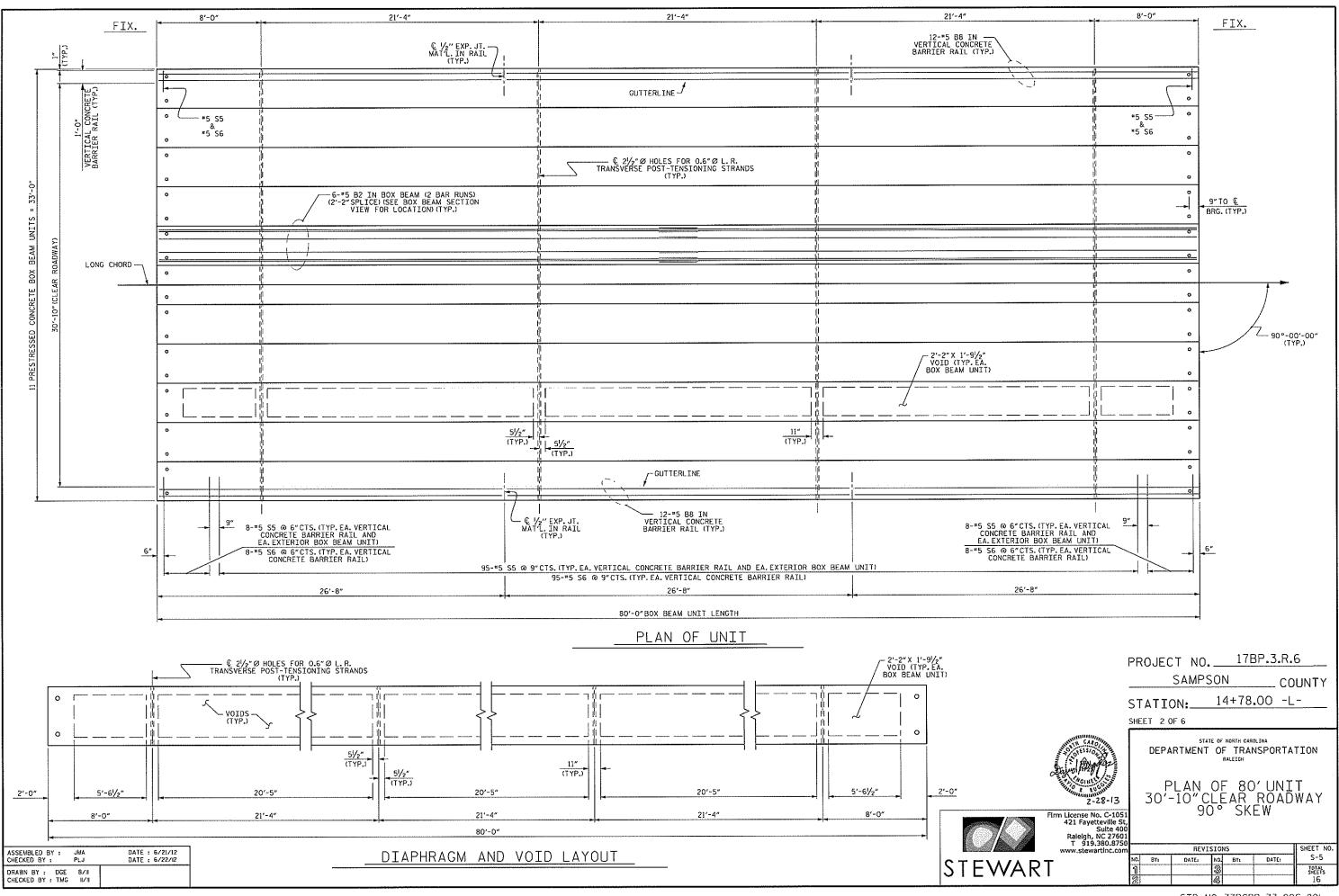
APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(8) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

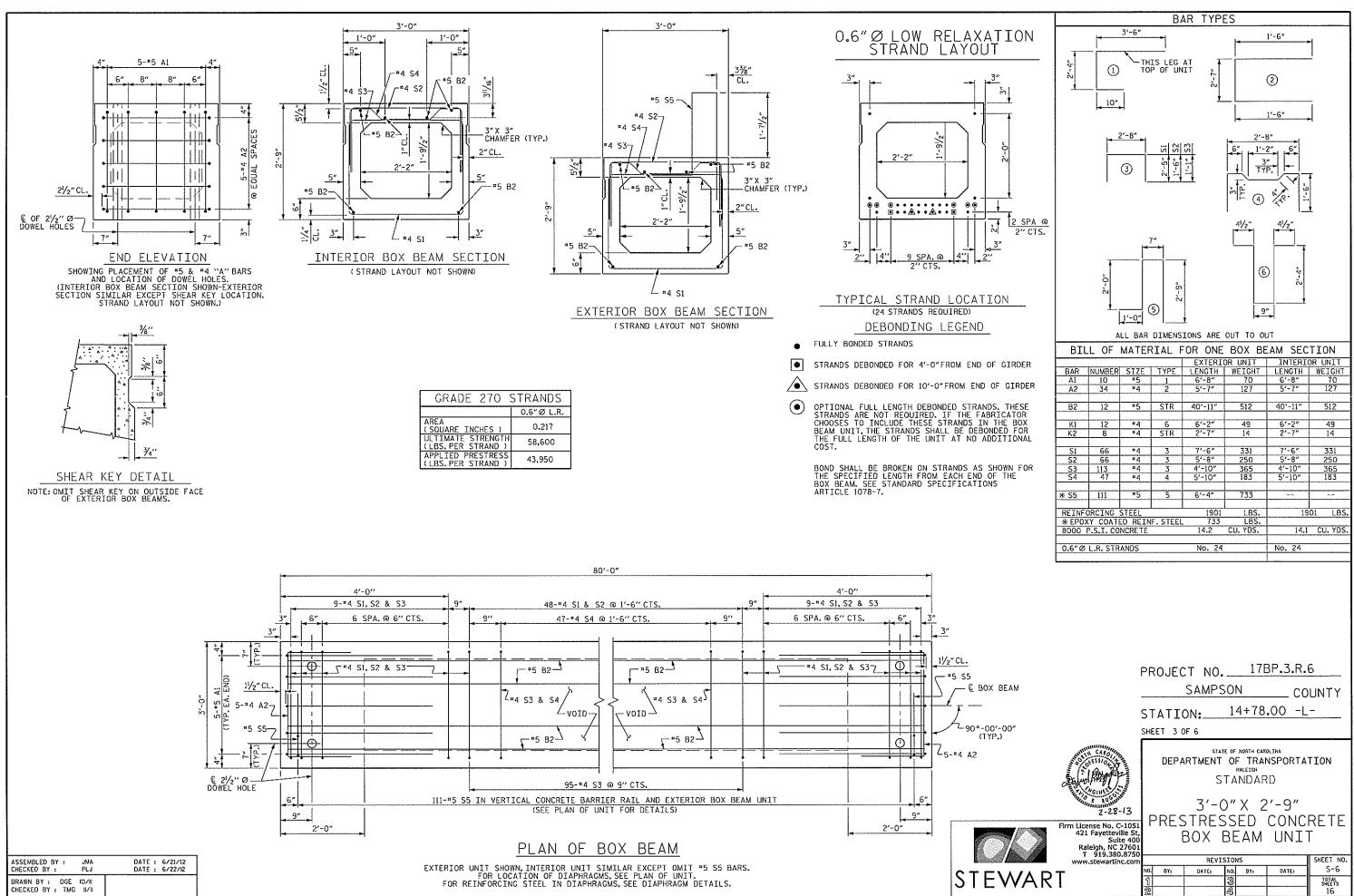
THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

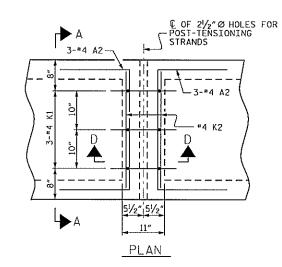
	PROJEC	T NO.	178	3P.3.R.	6
		SAMPS	ON	CO	UNTY
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	SHEET 1 OF	6			
	DEPAF	RTMENT	OF NORTH CAR OF TRAI RALEIGH	SPORTA	TION
2-28-13	PRES		)" X 2 SED	r-9" CONC	RETE
Firm License No. C-1051 421 Fayetteville St, Suite 400 Raleigh, NC 27601 T 919,380.8750				UNI	
www.stewartinc.com	· · ·	REVIS			SHEET NO.
ART	<u>NO. ВҮ.</u> 1 22		но. вт. 3 4	DATE:	S-4 Total Sheets 16
	STD.	NO. ST	D.33PC	BB_33.	.905

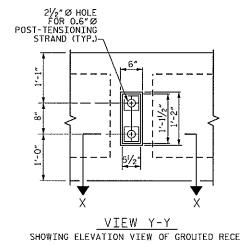


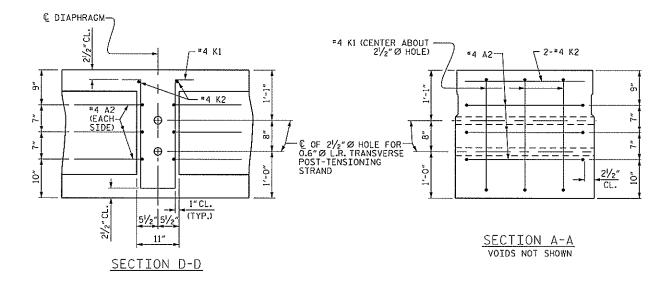
STD. NO. 33PCBB_33_90S_80L

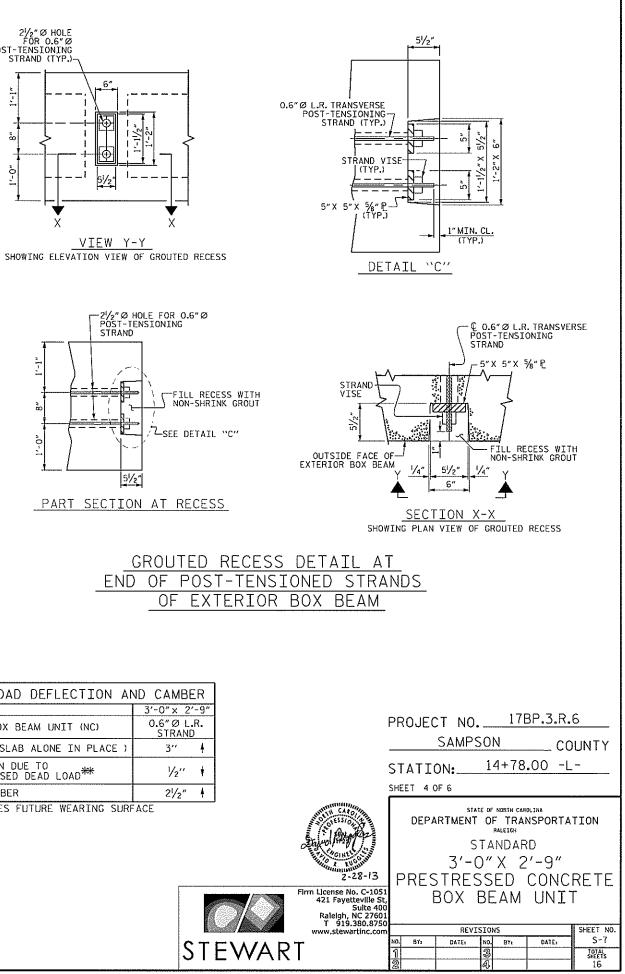


2 4 16 STD. NO. 33PCBB4_90S_80L



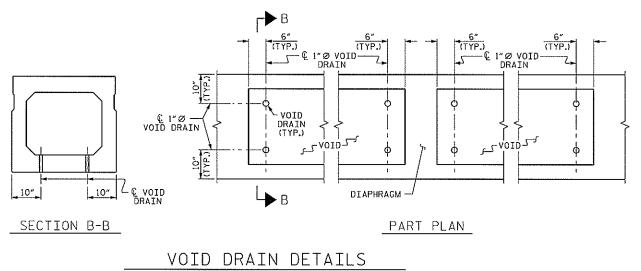




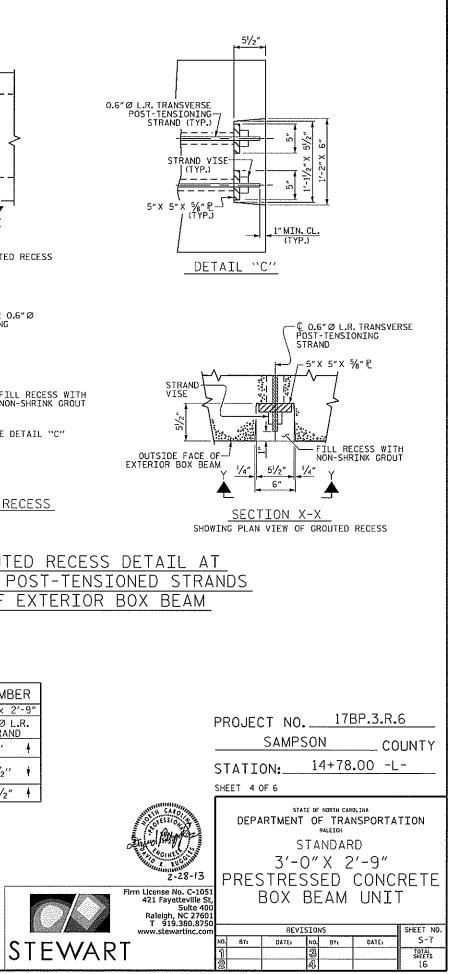


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END	0F	POS	T – T
	OF	F E.X	TER

## DOUBLE DIAPHRAGM DETAILS *4 "S" BARS NOT SHOWN. *4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR 2" Ø HOLE.

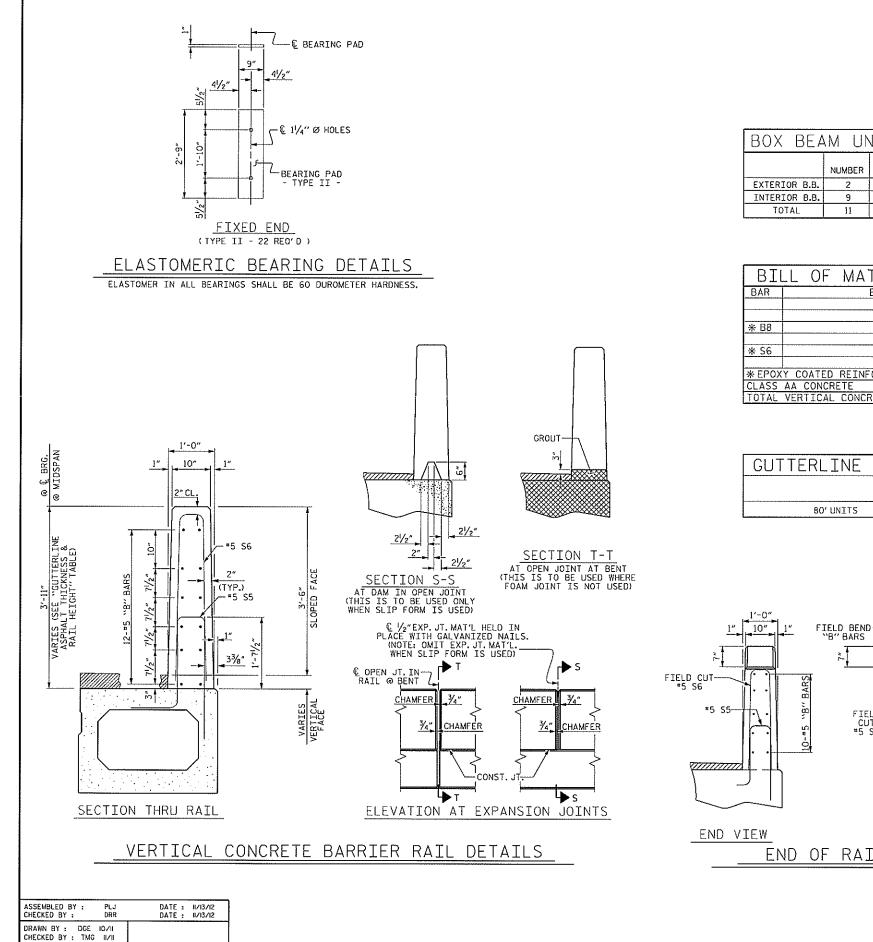


DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" x 2'-9"
80' BOX BEAM UNIT (NC)	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	3" 🕴
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	1/2" +
FINAL CAMBER	21/2" 🕴
** INCLUDES FUTURE WEARING SURF	ACE



		(DIMENSIONS	SHOWN /	ARE TYPICA	FOR EACH VOID
ASSEMBLED BY : JMA CHECKED BY : PLJ	DATE : 6/21/12 DATE : 6/22/12				
DRAWN BY : DGE IO/II CHECKED BY : TMG II/II					

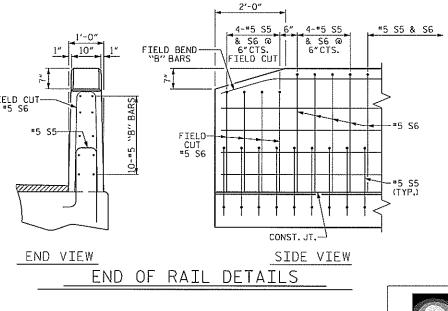
STD.NO.33PCBB5_90S

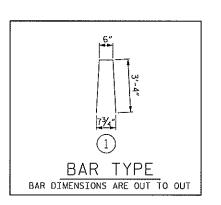


BOX BEA	AM UN	VITS RE	QUIRED
	NUMBER	LENGTH	TOTAL LENGTH
EXTERIOR B.B.	2	80'~0"	160'-0"
INTERIOR B.B.	9	80'-0"	720'-0"
TOTAL	11		880'-0"

BII	L OF MATERIAL FOR VERTICAL CONCRE	TE B	ARR	IER F	AIL	
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT	
	80' UNIT					
* B8	72	*5	STR	26'-3"	1971	
* S6	222	*5	1	7'-2"	1659	
* EPOX	* EPOXY COATED REINFORCING STEEL LBS.					
CLASS	CLASS AA_CONCRETE CU.YDS.					
TOTAL	VERTICAL CONCRETE BARRIER RAIL		LN.FT.		160.0	

GUTTERLINE	ASPHA	AL T	THICKNESS	&	RAIL
			PHALT OVERLAY THICKN @ MID-SPAN		
 80' UNITS			2"		

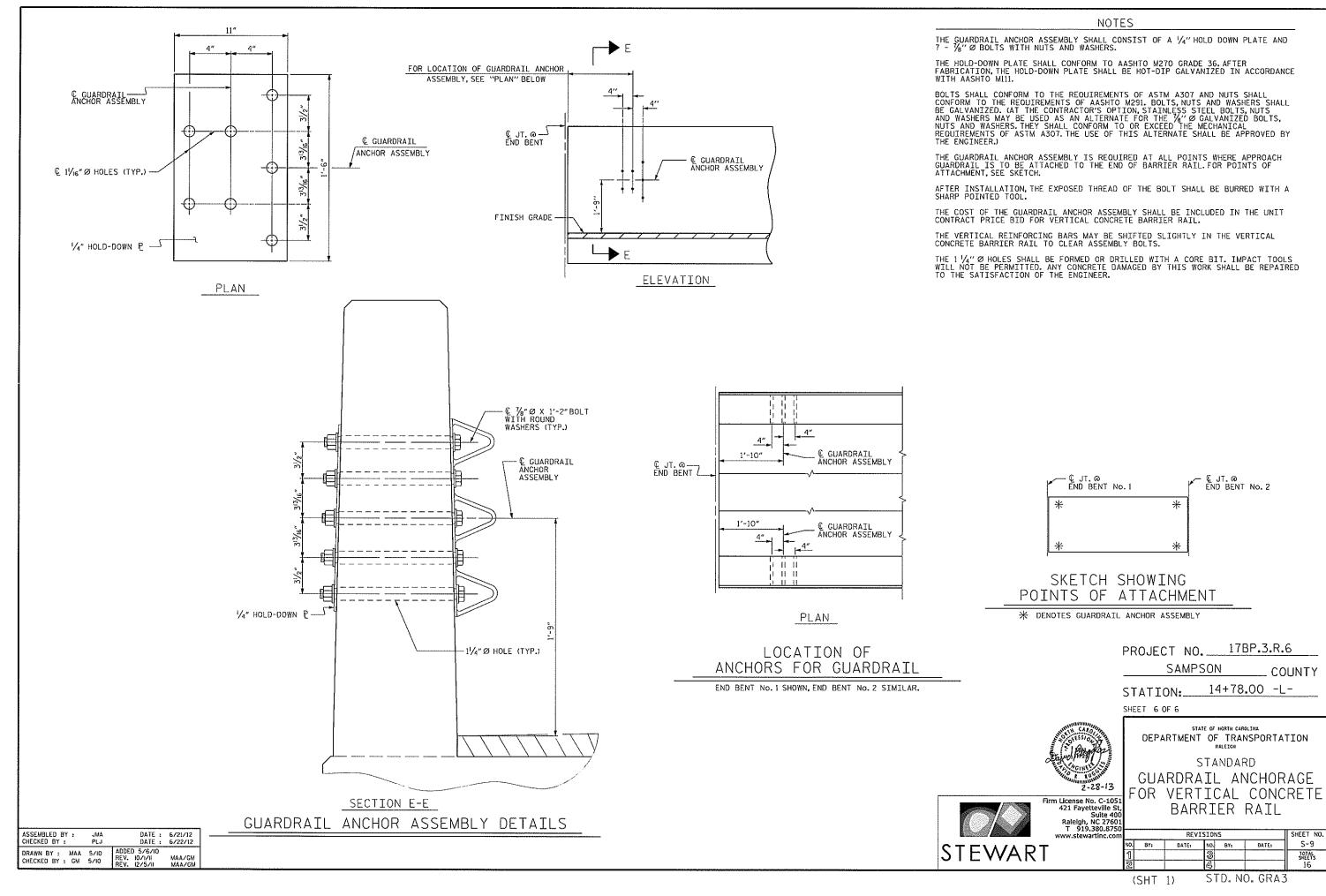




_ HEIGHT
RAIL HEIGHT @ MID-SPAN
3'-81/2"

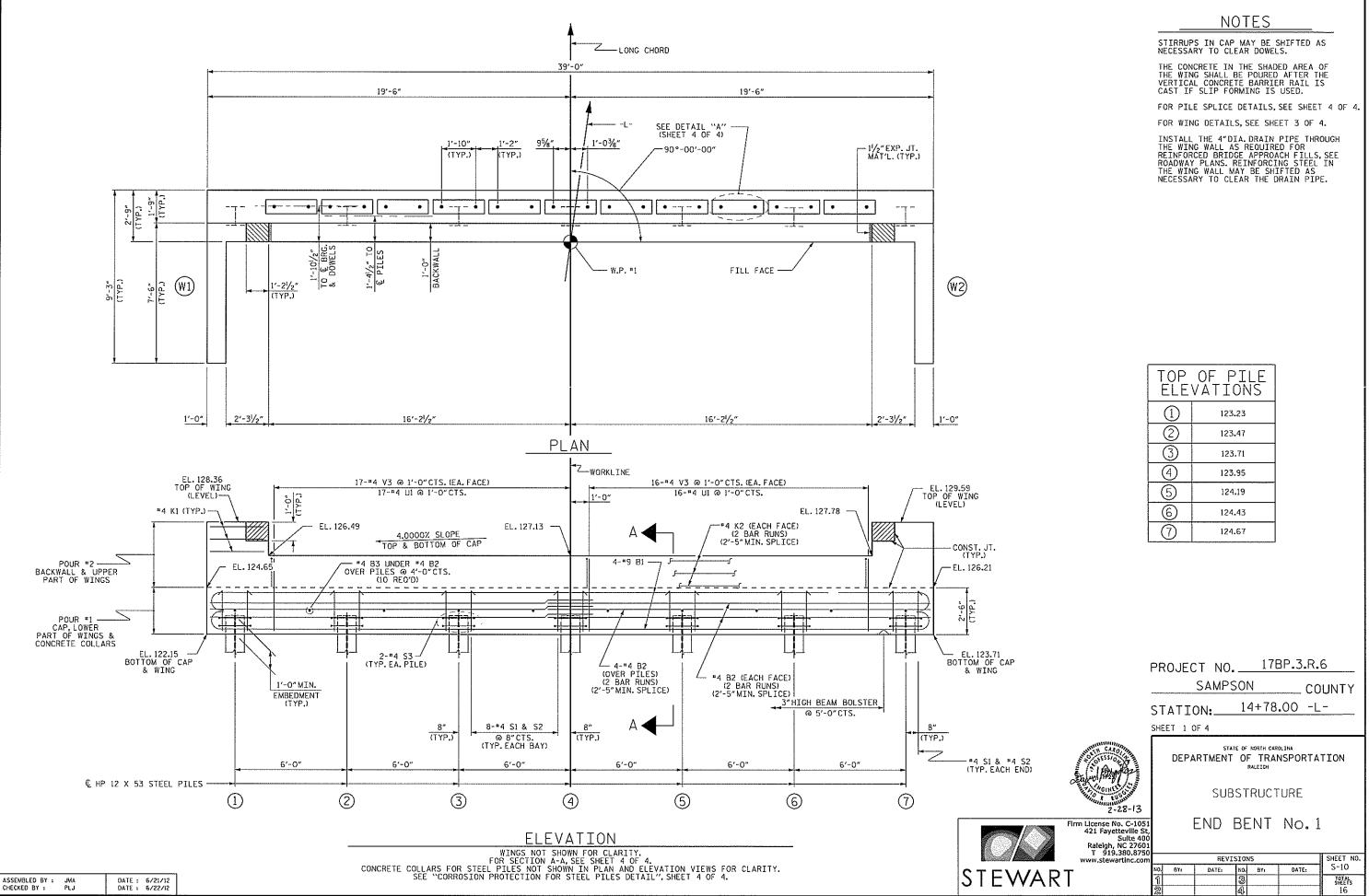
>	
*5 S6	PROJECT NO. <u>17BP.3.R.6</u>
	SAMPSONCOUNTY
	STATION: 14+78.00 -L-
	SHEET 5 OF 6
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEICH
2-28-13	3'-0" X 2'-9"
Firm License No. C-1051 421 Fayetteville St, Suite 400 Ralelgh, NC 27601	BOX BEAM UNIT
T 919.380.8750 www.stewartinc.com	REVISIONS SHEET NO.
STEWART	NO. BY: DATE: NO. BY: DATE: S-8 1 3 5014 2 4 16
	STD NO 33PCBB8 905

STD, NO, 33PCBB8_90S

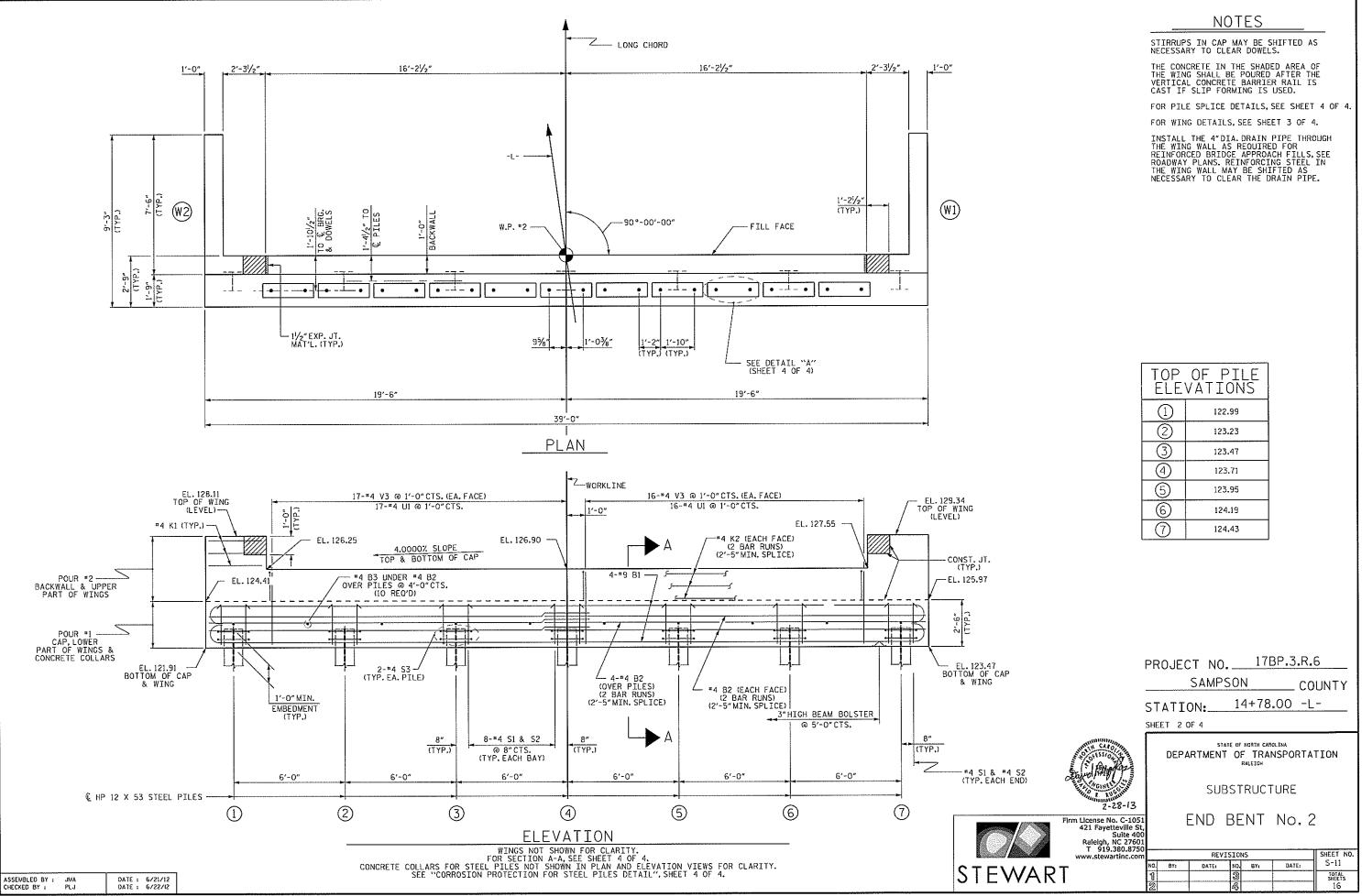


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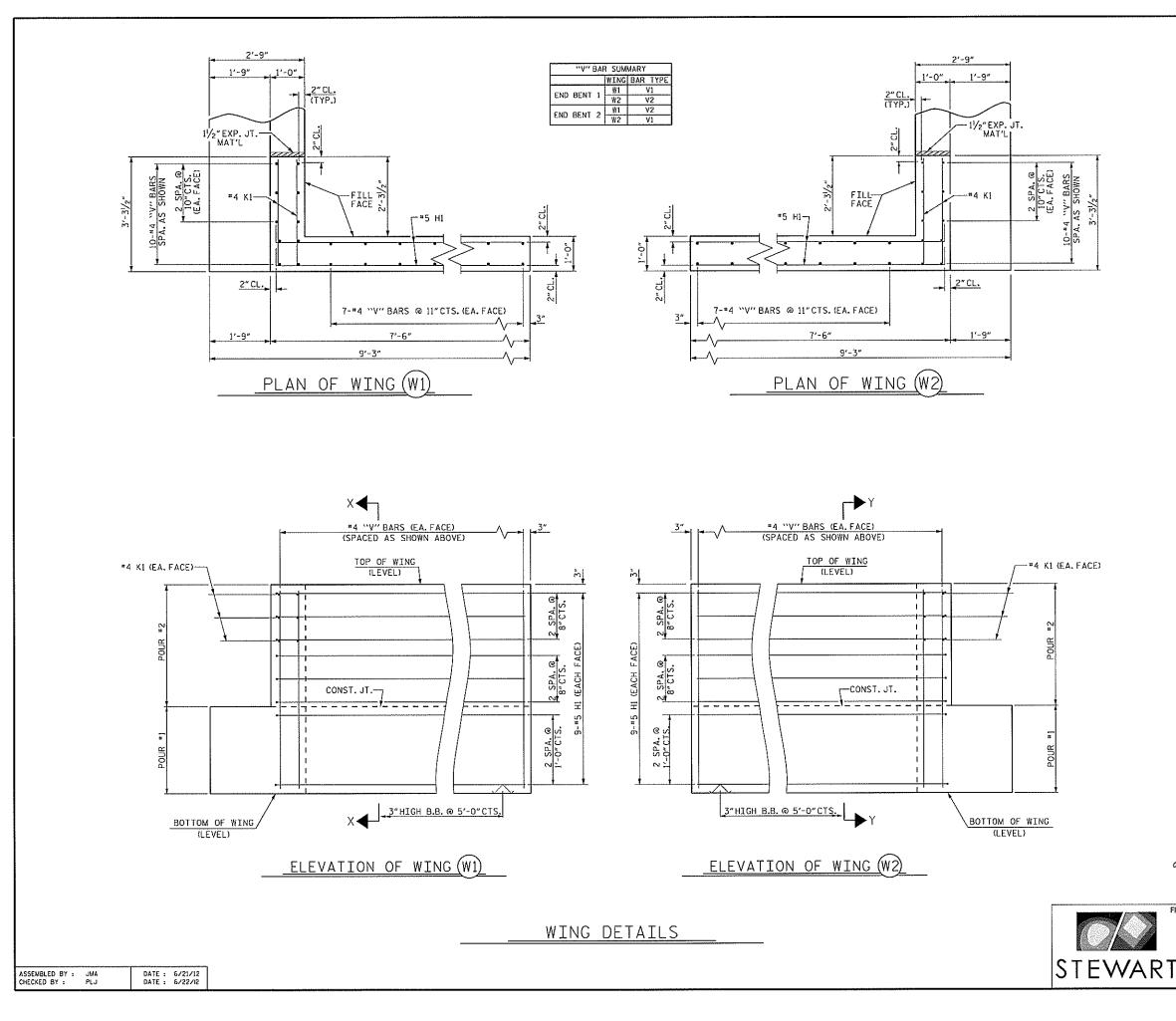


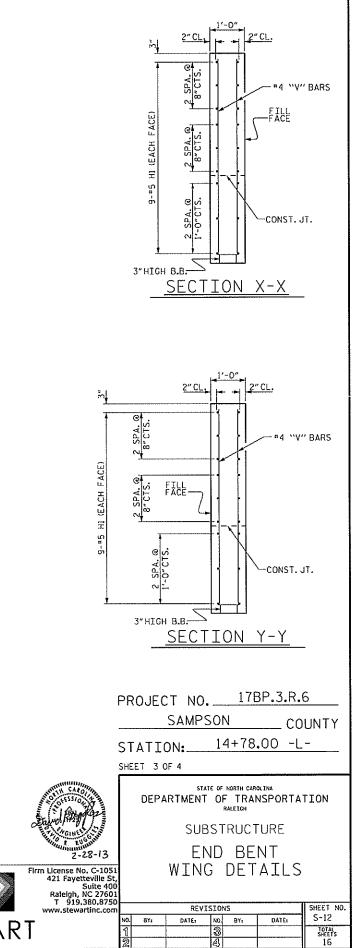
TOP ELE	OF PILE VATIONS
1	123.23
2	123.47
3	123.71
4	123.95
6	124.19
6	124.43
$\overline{)}$	124.67

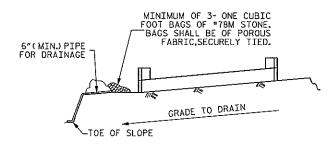


TOP OF PILE ELEVATIONS					
	122.99				
2	123.23				
3	123.47				
4	123.71				
5	123.95				
6	124.19				
7	124.43				

STD. NO. EB_33_90S4_33BB





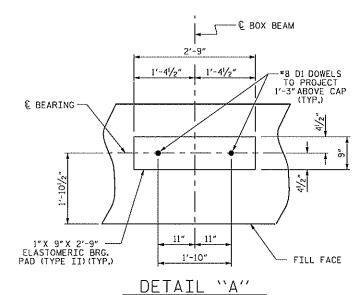


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

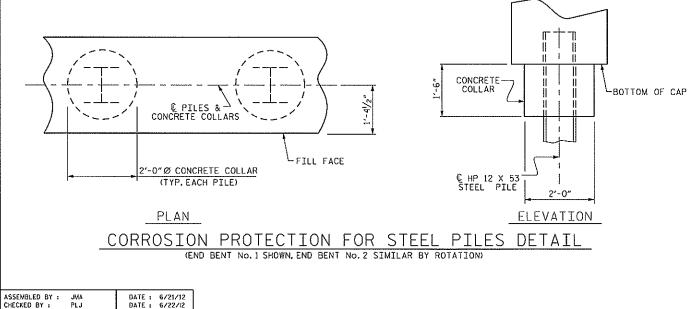
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

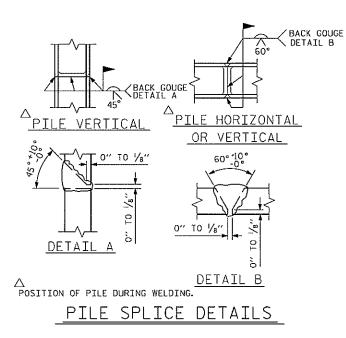
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

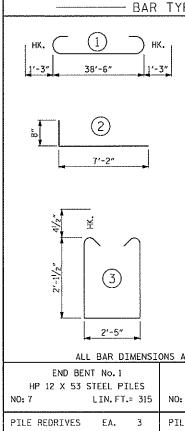
### TEMPORARY DRAINAGE AT END BENT



(END BENT No. 1 SHOWN, END BENT NO. 2 SIMILAR BY ROTATION)



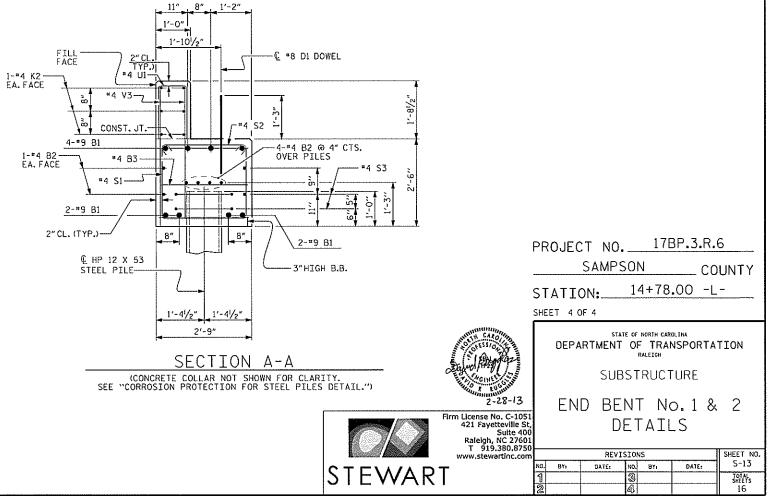


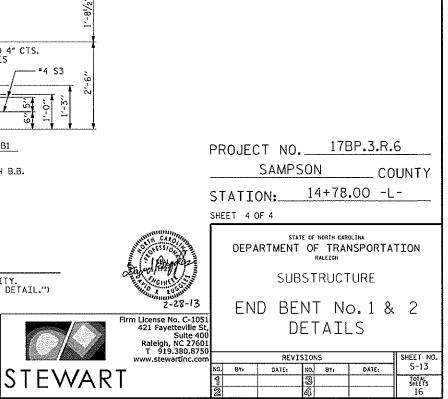


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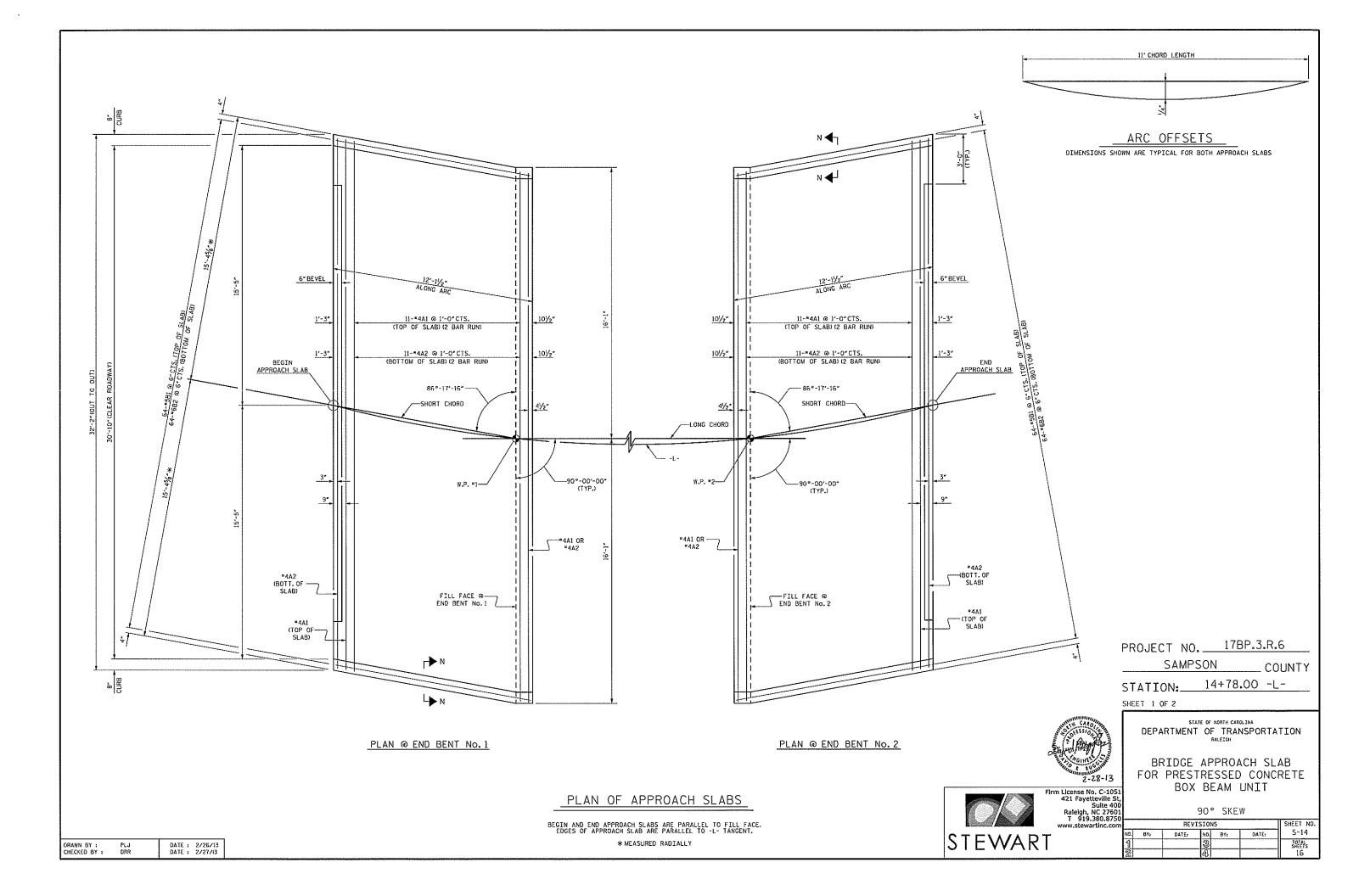
I

PDA TESTING





(PES	BILL OF MATERIAL					
	FOR ONE END BENT					
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
41/2" 2'-5" 41/2"	B1	8	#9	1	41'-0"	1115
	B2	16	#4	STR	20'-7"	220
' нк. ( <u>)</u> нк. '	B3	10	*4	STR	2'-5"	16
× (4)						
0	D1	22	*8	STR	2'-3"	132
I-J LAF	H1	36	*5	2	7'-10"	294
× ×			L.,			
	K1	12	*4	STR	2'-11"	23
	K2	12	#4	STR	20'-7"	165
(5)	<u>S1</u>	50	*4	3	7'-5"	248
	52 53	50	#4	4	3'-2"	106
		14	#4	<u> </u>	<u>6</u> 6.	61
1'-8" Ø	111	33	#4	6	3'-8"	81
		- 55	- 4	0	5-0	- 51
	V1	24	#4	STR	5'-10"	94
0.4	V2	24	=4	STR	5'-6"	88
<u>                                     </u>	V3	66	#4	STR	3'-10"	169
	RETNE	ORCTI	NG STE	FI		
			ND BEN			2812 LBS.
(a)			ONCRET		AKDOWN T)	
	POUR		AP,LOV F WINC		RT COLL ARS	12.4 C.Y.
ARE OUT TO OUT.						
END BENT No.2 HP 12 X 53 STEEL PILES	POUR		ACKWAL ART OF			4.6 C.Y.
11 12 × 35 57222 + 1225	ΤΟΤΑΙ	CLAS	SS A C	ONCRE	TE	17.0 C.Y.
LE REDRIVES EA. 3						

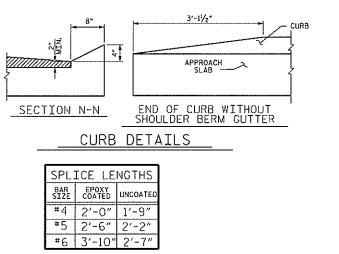


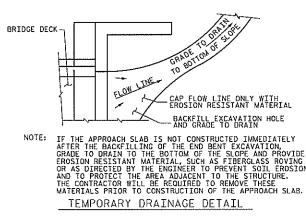


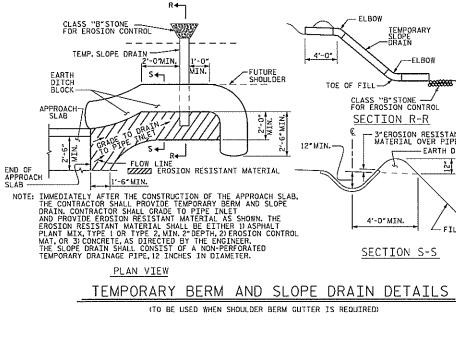
FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4°Ø DRAINAGE PIPE, *78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

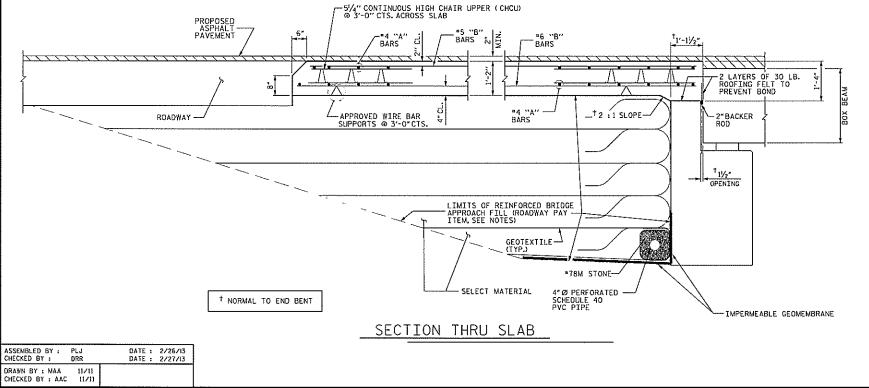
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.











BILL OF MATERIAL							
APPROACH SLAB AT EB #1						3 #1	
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
	* A1	26	=4	STR	16'-11"	294	
	A2	26	*4	STR	16'-9*	291	
Tent							
10 Stell 2Stell	* B1	64	*5	STR	11'-2"	745	
1. N. 31	B2	64	"6	STR	11'-8"	1121	
and of							
rto	REINF	ORCIN	IG STEE	L	L8S.	1412	
	* EPO REI		ATED ING ST	EEL	LBS.	1039	
LOW LINE ONLY WITH	CLASS AA CONCRETE C.Y. 17.2						
ON RESISTANT MATERIAL FILL EXCAVATION HOLE	APPROACH SLAB AT EB #2						
GRADE TO DRAIN	BAR	N0.	SIZE	TYPE	LENGTH	WEIGHT	
ONSTRUCTED IMMEDIATELY	* A1	26	=4	STR	16'-11″	294	
ND BENT EXCAVATION,	A2	26	=4	STR	16'-9"	291	
OF THE SLOPE AND PROVIDE UCH AS FIBERGLASS ROVING							
R TO PREVENT SOIL EROSION	* B1	64	#5	STR	11'-2"	745	
ENT TO THE STRUCTURE.	62	64	*6	STR	11'-8"	1121	

REINFORCING STEEL

CLASS AA CONCRETE

* EPOXY COATED REINFORCING STEEL

1412

1039

17.2

LBS.

L85.

C. Y.

$\overline{\}$	ELBOW SLOPE DRAIN	
FUTURE	TOE OF FILL	
NIW ,2 ,2 ATERIAL	CLASS "B" STONE FOR EROSION CONTROL SECTION R-R " " " " " " " " " " " " " " " " " "	
ROACH SLAB, ND SLOPE	Ī X	
DWN. THE ASPHALT ION CONTROL	4'-O" MIN.	
•	SECTION S-S	

	PROJEC	SAMPS	ON	3P.3.R. CO .00 -L	UNTY	
2-28-13	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE					
Firm License No. C-1051 421 Fayetteville St, Suite 400 Raleigh, NC 27601 T 919.380.8750		2011	BEAM 00° SKE			
www.stewartinc.com	COTT REVISIONS SHEET					
VART	<u>ко. вт.</u> 1 2	DATE:	<u>но, вт.</u> З 4.	DATE:	TOTAL SHEETS 16	

### STANDARD NOTES

### DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS. PER SO. IN.
- AASHTO M270 GRADE 50W -	27,000 LBS. PER SO. IN.
- AASHTO M270 GRADE 50 -	27,000 LBS. PER SO. IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS. PER SO. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SO. IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS. PER SO. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SO.IN.
EOUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REOUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REOUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT. ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOITOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ACTUAL BEAM CAMBER.

TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER. DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### **REINFORCING STEEL:**

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT

TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE %/" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0". EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EOUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EOUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EOUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SUBFACES WHICH BEAR ON OTHER SUBFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EOUIVALENT FLAT SUFFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

### SPECIAL NOTES:

SPECIFICATIONS ARTICLE 105-4.

ASSEMBLED BY : CHECKED BY :	JMA PL J		6/21/12 6/22/12
DRAWN BY : SHS/MAA CHECKED BY : BCH	5-09 5-09	REY. 12-11	WAA/AAC



METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS DTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OF OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE

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